I. GENERAL NOTES

A. THESE NOTES SHALL APPLY EXCEPT WHERE OTHERWISE INDICATED BY THE DRAWINGS OR SPECIFICATIONS.

B. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY MARKED ON THE DRAWINGS.

C. IF APPLICABLE, CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS OF EXISTING BUILDINGS AFFECTING NEW CONSTRUCTION, AND DISCREPANCIES ARE TO BE REPORTED IMMEDIATELY TO THE ARCHITECT/ENGINEER IN WRITING.

D. GENERAL CONTRACTOR SHALL ENSURE THAT ALL MATERIALS ARE IN COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.

E. ALL MASONRY WALLS BELOW GRADE SHALL BE BACKFILLED ON BOTH SIDES OF WALL SIMULTANEOUSLY. PROVIDE TEMPORARY BRACING AS REQUIRED TO ADEQUATELY SUPPORT STRUCTURE DURING CONSTRUCTION AND BACKFILLING. BRACING SHALL REMAIN IN PLACE UNTIL ALL FLOOR AND ROOF CONNECTIONS ARE COMPLETE.

F. CENTER LINE OF COLUMN = CENTER LINE OF FOOTING = CENTER LINE OF ANCHOR BOLT TEMPLATE UNLESS NOTED OTHERWISE. NO PIPING SHALL PASS THROUGH OR UNDER ANY FOOTING WITHOUT PRIOR APPROVAL OF THE ENGINEER.

G. DIMENSIONS AT FRAMED OPENINGS TO BE VERIFIED WITH APPLICABLE SUB-CONTRACTOR BEFORE FABRICATION OF STEEL. IF ANY DISCREPANCIES ARE FOUND, THE ARCHITECT/ENGINEER IS TO BE IMMEDIATELY NOTIFIED IN WRITING.

H. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO ORDERING MATERIAL. DISCREPANCIES BETWEEN FIELD MEASURMENTS OF THE EXISTING CONDITIONS AND THE DIMENSIONS INDICATED ON THE CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ENGINEER.

I. SUBMIT SHOP DRAWINGS OF ALL FABRICATED MATERIALS FOR REVIEW. CONTRACT DOCUMENTS REPRODUCED FOR USE AS SHOP DRAWINGS WILL BE RETURNED UNREVIEWED AND UNSTAMPED. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY ARE STAMPED "APPROVED" OR "APPROVED AS NOTED" BY THE GENERAL CONTRACTOR.

J. COORDINATE FOUNDATION TO ACCOMODATE DESIGN OF METAL BY OTHERS.

II. DESIGN CRITERIA:

A. BUILDING CODE : 2018 INTERNATIONAL BUILDING CODE

B. DESIGN LOADS:

1. ADDITIONAL DESIGN LOADS INDICATED ON STRUCTURAL DRAWINGS SHALL BE IDENTIFIED AS FOLLOWS:

   LL = LIVE LOAD
   WL = WIND LOAD
   EL = SEISMIC LOAD
   SN = SNOW LOAD
   SD = SNOW DRIFT

2. WIND:
   a. V_{ult} = 105 mph
   b. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   c. MEAN ROOF HT = 22'-6"
   d. EXPOSURE B
   e. END ZONE = 5'-0"
   f. ENCLOSED BUILDING, GC_{pi}=+/-0.18
   g. WIND PRESSURES FOR COMPONENTS AND CLADDING BY TRIBUTARY AREA. VALUES BASED ON WINDSPEED=105 MPH AND EXPOSURE=B. VALUES ARE UNFACTORED AND MAY BE USED IN EITHER STRENGTH DESIGN OR ALLOWABLE STRESS DESIGN LOAD COMBINATIONS OF ASCE 7-10.

	TRIBUTARY AREA			
REGION	10 ft²	20 ft²	50 ft²	100 ft²
1	+11.2/-40.8	+9.2/-40.8	+8.2/-22.4	+6.1/-10.2
2e	+11.2/-40.8	+9.2/-40.8	+8.2/-22.4	+6.1/-10.2
2n	+11.2/-61.2	+9.2/-53.0	+8.2/-40.8	+6.1/-32.6
2r	+11.2/-61.2	+9.2/-53.0	+8.2/-40.8	+6.1/-32.6
3e	+11.2/-61.2	+9.2/-53.0	+8.2/-40.8	+6.1/-32.6
3r	+11.2/-73.4	+9.2/-61.2	+8.2/-49.0	+6.1/-36.7
4	+20.4/-22.4	+19.4/-20.4	+17.3/-20.4	+16.3/-18.4
5	+20.4/-28.6	+19.4/-26.5	+17.3/-24.5	+16.3/-20.4
LOADS TO BE APPLIED PER FIGURES 30.3-1 AND 30.3-2B OF ASCE 7-16.				

3. SEISMIC:
   a. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   b. SITE CLASS = D
   c. S_s = 0.52 S_{ds} = 0.48
      S_1 = 0.122 S_{d1} = 0.192
   d. SEISMIC DESIGN CATEGORY = C
   e. EQUIVALENT LATERAL FORCE PROCEDURE
   f. STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE R=3
   g. C_s = 0.16
   h. DESIGN BASE SHEAR = 8.0 "

4. SNOW:
   a. RISK CATEGORY = II IMPORTANCE FACTOR = 1.00
   b. Pg = 10 psf Pf = 7 psf
   c. Ce = 1.0 Ct= 1.0

C. FOUNDATIONS:

1. FOUNDATION DESIGN IS BASED ON SUBSURFACE EXPLORATION REPORT PREPARED BY: TERRACON

2. IF, AFTER EXCAVATION, THE CONDITION OF THE SOIL INDICATES A SAFE BEARING CAPACITY OF LESS THAN 2000 p.s.f. THE ENGINEER SHALL BE NOTIFIED AND THE FOOTINGS REVISED IF NECESSARY.

3. ALL GRADING AND FILLING SHALL BE DONE AS RECOMMENDED BY A GEOTECHNICAL ENGINEER. GENERAL CONTRACTOR SHALL ESTABLISH AND MAINTAIN SITE DRAINAGE TO DIRECT WATER AWAY FROM FOOTING EXCAVATIONS AND FILL PLACEMENT.

4. ALL FOOTINGS SHALL BE POURED ON FIRM, UNDISTURBED EARTH OR ENGINEERED CONTROLLED BACKFILL. BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 18 INCHES BELOW FINISH GRADE UNLESS OTHERWISE NOTED, AND TOP OF FOOTING MUST COURSE WITH FINISH FLOOR. FROST LINE FOR THIS PROJECT IS 12 BELOW FINISH GRADE.

III. CONCRETE:

A. CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

B. CEMENT TO BE PORTLAND TYPE 1 OR APPROVED EQUAL.

C. MIX DESIGN SHALL BE DOCUMENTED IN ACCORD WITH SECTION 03300 OF THE PROJECT SPECS AND ACI 301, CHAPTER 3 "PROPORTIONING". MIX DESIGNS WHICH ARE SUBMITTED WITHOUT THE REQUIRED DOCUMENTATION WILL BE REJECTED. FIELD SLUMPS RECORDED AT JOB SITE SHALL NOT EXCEED THE SLUMP ESTABLISHED FOR THE MIX DESIGN.

D. CONCRETE SHALL HAVE AN ALLOWABLE COMPRESSIVE STRENGTH AS NOTED BELOW:

   * INTERIOR SLABS ON GRADE, F'c = 3,000 PSI
   FOUNDATIONS F'c = 3,000 PSI

E. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

   FOOTINGS 3"

F. EXPOSED CONCRETE CORNERS SHALL BE CHAMFERED 3/4 INCH UNLESS NOTED.

G. CONCRETE SHALL NOT BE POURED IN WATER OR ON FROZEN GROUND AND SHALL BE PROTECTED FROM FROST DURING CONSTRUCTION.

H. CONTRACTOR SHALL COORDINATE ALL CONTRACT DRAWINGS FOR THE LOCATION OF ANCHOR BOLTS, FLOOR DRAINS, INSERTS, ETC., BEFORE POURING CONCRETE.

I. SLABS:

1. SLAB THICKNESS INDICATED ON DRAWINGS IS MINIMUM AND SHALL BE MEASURED FROM LOW POINT ON FLOOR. CONTRACTOR SHALL COORDINATE ALL DRAWINGS TO ASSURE THAT ALL FLOORS HAVE PROPER SLOPE TO DRAIN IN TOILETS, SHOWERS, ETC.

2. "C.J.", AS INDICATED ON SLAB, INDICATES 3/4" DEEP SAW CUT CONTROL JOINT OR KEYED CONSTRUCTION JOINT IN SLAB-ON-GRADE. MAKE CUTS WITHIN 12 HOURS AFTER CONCRETE PLACEMENT.

J. REINFORCEMENT:

1. ALL DETAILING, FABRICATION AND PLACEMENT OF REINFORCING STEEL, MIXING, HANDLING, PLACING, FINISHING AND CURING OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI-315 AND ACI-318.

2. WIRE MESH REINFORCEMENT SHALL BE CENTERED IN SLAB, BY USE OF HIGH CHAIR WELDING WASHERS OR CONTINUOUS BEAM BOLSTERS.

3. WELDED WIRE FABRIC AND WIRE SHALL BE LAPPED THE SPACING OF THE CROSS WIRES PLUS 2".

IV. STRUCTURAL STEEL:

A.. DESIGN, FABRICATION, & ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE MANUAL OF STEEL CONSTRUCTION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION NINTH EDITION UNLESS OTHERWISE MODIFIED ON THE DRAWINGS OR IN SPECIFICATIONS.

B. MATERIAL SHALL MEET THE REQUIREMENTS OF THE FOLLOWING SPECIFICATION UNLESS NOTED:

STRUCTURAL STEEL	- ASTM A992 (Fy=50 ksi)
ROUND PIPE STEEL	- ASTM A53 GRADE B (Fy=35ksi)
TUBE STEEL	- ASTM A500 GRADE B (Fy=46ksi)
HIGH STRENGTH BOLTS	- ASTM A325-86A, 3/4"Ø
WELD STEEL	- AWS CLASS E70

C. ALL SHOP CONNECTIONS SHALL BE WELDED OR MADE WITH HIGH STRENGTH BOLTS UNLESS NOTED SPECIFICALLY.

D. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" HIGH STRENGTH BOLTS. FIELD WELDING WILL BE ALLOWED ONLY WHERE NOTED ON THE DRAWINGS AND DETAILS.

E. ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE BEARING TYPE WITH THREADS INCLUDED IN THE SHEAR PLANE UNLESS NOTED.

F. ALL HIGH STRENGTH FIELD BOLTED CONNECTIONS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD AS SPECIFIED IN "THE ALLOWABLE STRESS DESIGN SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."

G. FRAMED BEAM CONNECTIONS SHALL DEVELOP THE REACTION SHOWN ON ENDS OF BEAMS ON STRUCTURAL PLANS. WHERE REACTIONS ARE NOT SHOWN THE CONNECTION SHALL DEVELOP ONE-HALF THE ALLOWABLE UNIFORM LOAD FOR LATERALLY SUPPORTED BEAMS AS SHOWN IN TABLES IN PART 2 OF THE AISC MANUAL. CONNECTION DESIGN TO BE SUBMITTED BY ENGINEER LICENSED IN THE PROJECT STATE.

H. ALL STRUCTURAL STEEL BELOW GRADE SHALL BE ENCASED WITH A MINIMUM OF 4" CONCRETE COVER OR PAINTED WITH A COAL TAR

V. METAL STUDS:

1. UNLESS NOTED OTHERWISE THE FOLLOWING SHALL APPLY
2. MINIMUM YIELD STRENGTH FOR 20ga AND 18ga STUDS SHALL BE 33 ksi MINIMUM YIELD STRENGTH FOR 16ga, 14ga AND 12ga STUDS SHALL BE 50 ksi - ALL TRACKS SHALL BE 33 ksi - WIND STRAPS ARE BASED ON THE USE OF 40,000 psi FY MIN
3. ALL STUDS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING MEETING THE REQUIREMENTSOF A.S.T.M. A525.
4. BOTH STUD FLANGES MUST BE FASTENED TO TRACK AT TOP AND BOTTOM AS SHOWN ON DRAWINGS.
5. PANELS MUST BE FABRICATED WITH WELDS. WELDS SHALL BE PERFORMED OPERATORS QUALIFIED IN ACCORDANCE WITH SECTION 6.0 OF THE AMERICAN WELDING SOCIETY'S "STRUCTURAL WELDING CODE - SHEET METAL (AWS D1.3-8)
6. ALL WELDS SHALL BE TOUCHED UP WITH ZINC RICH PAINT.
7. STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO ATTACHMENT AT BOTH ENDS - NO SPLICES, CUTS OR NOTCHES PERMITTED AT LOAD BEARING STUDS.
8. AT TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY SHALL BE BUTT-WELDED OR SPLICED TOGETHER
9. A MINIMUM OF 10" OF UN-PUNCHED STEEL IS REQUIRED AT BOTH ENDS OF STUDS
10. BRIDGING SHALL BE 1.5" CRC PLACED THROUGH PUNCH OUTS AND WELDED ON BOTH SIDES BRIDGING IS TO BE SPACED AT NO MORE THAN 3'-6" VERTICALLY AT APPROXIMATELY THE THIRD POINT - CRC BRIDGING IN 6" STUDS REQUIRE A CLIP ANGLE AT EACH CONNECTION LOCATION
11. USE THREE (3) STUDS AT THE CORNER OF ALL EXTERIOR WALLS USE THREE (3) STUDS AT THE INTERSECTION OF ALL LOAD BEARING WALLS (EXTERIOR AND/OR INTERIOR)
12. ALL MULTIPLE STUD GROUPS SHALL BE WELDED TOGETHER WITH 1" OF WELD @ 12"o.c. MAX ON BOTH FACES.
13. TRACKS SHOWN @ BOTTOM OF DOOR OPENINGS ARE FOR EASE OF HANDLING AND MAY BE REMOVED IN AREA OF DOOR AFTER PANEL IS ERECTED
14. THE DRYWALL SUBCONTRACTOR SHALL BE RESPONSIBLE FOR ADDING ANY METAL STUDS, BLOCKING, OR FURRING THAT MAY BE NEEDED FOR DRYWALL ATTACHMENT
15. ALL STUD WALLS SHALL EXTEND TO BOTTOM OF STRUCTURE ABOVE.
16. STUDS MUST BE INSTALLED UNDER ALL JOIST BEARING LOCATIONS.
17. MINIMUM STRUCTURAL PROPERTIES LISTED IN THE LIGHT GAGE FRAMING SCHEDULE SHALL APPLY. SEE SCHEDULE THIS SHEET.
18. CONTRACTOR TO SUBMIT LIGHT GAUGE SHOP DRAWINGS STAMPED AND SIGNED BY P.E. LICENSED IN STATE OF GEORGIA.

VI. FASTENERS:

EXCEPT WHERE SPECIFICALLY NOTED OTHERWISE ON DRAWINGS:

A. ALL POWDER ACTUATED FASTENERS (P.A.F.) TO BE .145" SHANK DIAMETER x 1 1/4" LONG HILTI X-DNI 32 P8 OR .177" SHANK DIAMETER x 1 1/4" LONG HILTI DS 37 P10, TYP. U.N.O.

B. ALL EXPANSION ANCHORS TO BE HILTI KWIK BOLT II
   5/8"Ø - MIN. EMBED = 4"
   3/4"Ø - MIN. EMBED = 4 3/4"
   1"Ø - MIN. EMBED = 6"

C. ALL SLEEVE ANCHORS TO BE HILTI CARBON STEEL SLEEVE ANCHORS.
   1/2"Ø - MIN. EMBED = 1 1/2"

D. ALL EPOXY ANCHORS TO BE HILTI HIT HY200 ADHESIVE ANCHORS.
   5/8"Ø - MIN. EMBED = 5"
   3/4"Ø - MIN. EMBED = 6 5/8"
   7/8"Ø - MIN. EMBED = 7 1/2"
   1"Ø - MIN. EMBED = 8 1/4"

E. ALL CONCRETE/MASONRY SCREWS TO BE HILTI KWIK-CON II+.
   3/16"Ø - MIN. EMBED = 1"
   1/4"Ø - MIN. EMBED = 1 3/4"

F. ALL FASTENERS ARE SIZED PER HILTI SPECIFICATIONS. ALL FASTENERS MAY BE SUBSTITUTED BY AN EQUIVALENT THAT MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

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VII. WOOD FRAMING NOTES

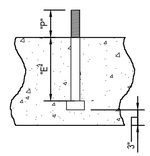
1. WALL STUDS SHALL BE DOUBLED AT ALL ANGLES, CORNERS, AND AROUND ALL OPENINGS.
2. REFER TO PLAN FOR ALL SHEAR WALL LOCATIONS.
3. PROVIDE ALL BLOCKING AND FIRE STOPS REQUIRED BY THE BUILDING OFFICIAL.
4. UNLESS OTHERWISE NOTED, ALL TIMBER CONNECTIONS SHALL BE NAILED IN CONFORMANCE WITH THE 1997 UNIFORM BUILDING CODE.
5. FLOOR SHEATHING SHALL BE DOUGLAS FIR-LARCH 3/4" TONGUE AND GROOVE PLYWOOD (C-D OR C-C) - NAILS SHALL BE 10d WITH MIN OF 1 1/2" PENETRATION IN FRAMING - PLYWOOD SHALL BE BLOCKED WITH CONTINUOUS PANEL NAIL SPACING OF 4" (UNO) ALONG BEARING WALLS, SHEAR WALLS AND CORRIDORS - NAIL SPACING AT OTHER PANEL EDGES SHALL BE 6" (UNO)
6. ROOF SHEATHING SHALL BE DOUGLAS FIR-LARCH 5/8" PLYWOOD (C-D OR C-C) NAILS SHALL BE 8d WITH MIN OF 1 1/2" PENETRATION IN FRAMING NAILS SHALL BE SPACED @ 6"o/c AT SUPPORTED EDGES

MINIMUM WOOD PROPERTIES

	DIMENSIONAL LUMBER					LAMINATED VENEER LUMBER (LVL)	PARALLEL STRAND LUMBER (PSL)
	#2 SOUTHERN YELLOW PINE (SYP)						
	2x4	2x6	2x8	2x10	2x12		
Fb	1,500 psi	1,250 psi	1,200 psi	1,050 psi	975 psi	2,925 psi	2,400 psi
Fv	175 psi	175 psi	175 psi	175 psi	175 psi	285 psi	190 psi
$Fc_{\perp}$	1,650 psi	1,600 psi	1,550 psi	1,500 psi	1,450 psi	1,600 psi	2,500 psi
$Fc_{\perp}$	565 psi	565 psi	565 psi	565 psi	565 psi	750 psi	545 psi
E	1,600,000 psi	1,600,000 psi	1,600,000 psi	1,600,000 psi	1,600,000 psi	2,000,000 psi	1,800,000 psi

ANCHOR BOLT EMBEDMENTS SCHEDULE

DIAMETER	GRADE	"P"	"E"
1/2"	F1554 (36KSI)	6"	10"
5/8"	F1554 (36KSI)	6"	10"
3/4"	F1554 (36KSI)	6"	10"
7/8"	F1554 (36KSI)	6"	11"
1"	F1554 (36KSI)	6"	12"
1 1/8"	F1554 (36KSI)	6"	14"
1 1/4"	F1554 (36KSI)	6"	15"



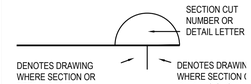
SCHEDULE NOTES:

1. ALL COLUMNS TO BE CONTINUOUS U.N.O.

COLUMN SCHEDULE

MARK	C1	C2	C3
COLUMN	HSS6"x6"x1/4"	HSS10"x10"x1/4"	HSS4"x4"x1/4"
BASE PLATE	3/4"x12"x1'-0"	3/4"x16"x1'-4"	3/4"x10"x0'-10"
ANCHOR BOLT	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1
NOTES			

LEGEND



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STRUCTURAL NOTES

SHEET NO.

S0.1

VIII. SPECIAL INSPECTIONS:

THE FOLLOWING SPECIAL INSPECTIONS SHALL BE PERFORMED PER CHAPTER 17 OF THE 2018 INTERNATIONAL BUILDING CODE:

GENERAL NOTES:

1. INSPECTION OF FABRICATORS: SPECIAL INSPECTOR IS TO VERIFY THAT FABRICATORS MAINTAIN DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO THE APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. SEE 1704.2.5.1.
2. REFERENCE CURRENT STATE BUILDING CODE FOR APPLICABLE REFERENCED STANDARD SPECIFIC EDITION INFORMATION.
3. CONTRACTOR RESPONSIBILITY:
   a. EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND- OR SEISMIC-FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC-RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.

IBC 2018 SECTION 1705.2
REQUIRED SPECIAL INSPECTIONS AND TESTS OF STEEL CONSTRUCTION

1. SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY CONTROL AND QUALITY ASSURANCE REQUIREMENTS OF THE LATEST EDITIONS OF AISC 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND THE COMMENTARY OF AISC 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS .
2. SPECIAL INSPECTIONS AND QUALIFICATION OF WELDING SPECIAL INSPECTORS FOR COLD-FORMED STEEL FLOOR AND ROOF DECK SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF SDI QA/QC.

IBC 2018 TABLE 1705.3
REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESING TENDONS, AND PLACEMENT.	-		ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A-706 b. INSPECT SINGLE PASS FILLET WELDS, MAXIMUM 5/16" c. INSPECT ALL OTHER WELDS	- - X	X X -	AWS D1.4 ACI 318: 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE.	-	X	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A.	X -	- X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C 172 ASTM C 31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	ACI 318: 26.5	1908.6, 1908.7 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES; AND b. GROUTING OF BONDED PRESTRESSING TENDONS.	X X	- -	ACI 318: 26.10	
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	ACI 318: Ch. 26.9	
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 26.11.2	
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 26.11.1.2(b)	

IBC 2018 TABLE 1705.6
REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

VERIFICATION AND INSPECTION	CONTINUOUSLY DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

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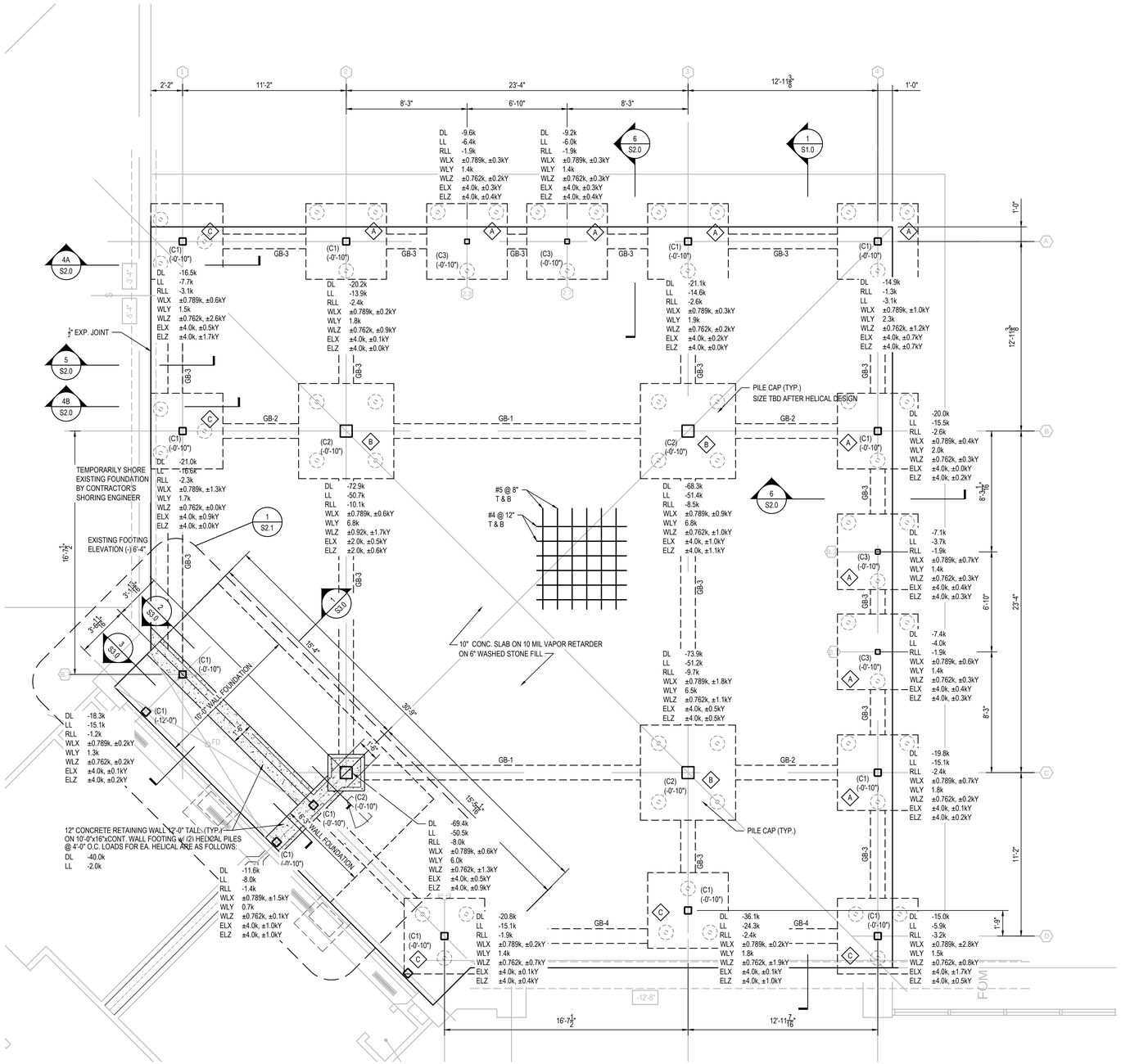


LEGEND

DL	- DEAD LOAD
LL	- LIVE LOAD
RLL	- ROOF LIVE LOAD
WLX	- WIND LOAD X-DIRECTION
WLY	- WIND LOAD Y-DIRECTION
WLZ	- WIND LOAD Z-DIRECTION
SL	- SNOW LOAD

+Y (UP), +X, +Z

NOTE: SNOW LOAD REACTIONS ARE EQUAL TO $\frac{1}{2}$ EACH RLL REACTION
ALL REACTIONS ARE UNFACTORED

STRUCTURAL NOTES:

1. ALL PILE CAPS SHALL BE CENTERED UNDER COLUMNS AND BEAM BEARING WALL REINFORCEMENTS U.N.O.
2. SEE SHEET S2.1 FOR ADDITIONAL SECTIONS AND DETAILS
3. ALL SECTIONS TYP U.N.O.

STRUCTURAL LEGEND:

(?) COLUMN DESIGNATION - SEE COLUMN SCHEDULE THIS SHEET

SCHEDULE NOTES:

1. ALL COLUMNS TO BE CONTINUOUS U.N.O.

COLUMN SCHEDULE

MARK	C1	C2	C3
COLUMN	HSS6"x6"x1/4"	HSS10"x10"x1/4"	HSS4"x4"x1/4"
BASE PLATE	3/4"x12"x1'-0"	3/4"x16"x1'-4"	3/4"x10"x0'-10"
ANCHOR BOLT	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1	3/4" HEAVY HEX BOLT SEE SCHEDULE DWG. S0.1
NOTES			

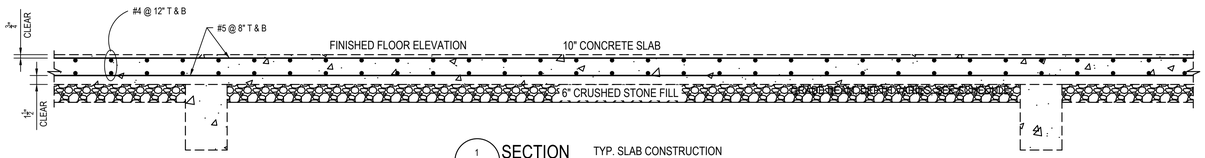
PILE CAP TYPES

MARK	CAPACITY	NO. OF PILES	MAX. LOAD	DETAIL
A	40 TON	3	-k	1/ S2.0
B	50 TON	4	-k	2/ S2.0
C	40 TON	3	-k	3/ S2.0, 4/ S2.0

GRADE BEAM SCHEDULE

MARK	GB-1	GB-2	GB-3	GB-4
BEAM SIZE (W x D)	14W x 22D	14W x 22D	12W x 22D	14W x 30D
REINFORCEMENT	(3) #8 T & B	(4) #6 TOP (2) #6 BOTTOM	(2) #6 T & B	(3) #7 T & B
NOTES	#4 STIRRUPS @ 9" O.C. 3" CLEAR COVER BOTTOM 3/4" CLEAR COVER TOP	#4 STIRRUPS @ 9" O.C. 3" CLEAR COVER BOTTOM 3/4" CLEAR COVER TOP	#4 STIRRUPS @ 9" O.C. 3" CLEAR COVER BOTTOM 3/4" CLEAR COVER TOP	#4 STIRRUPS @ 12" O.C. 3" CLEAR COVER BOTTOM 3/4" CLEAR COVER TOP

1 S1.0 FOUNDATION PLAN
SCALE: 1/4"=1'-0"
NORTH



1 S1.0 SECTION TYP. SLAB CONSTRUCTION
SCALE: 1/2"=1'-0"

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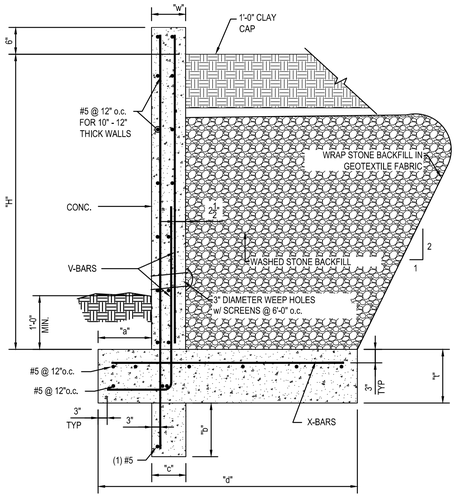
DRAWING TITLE
FOUNDATION PLAN

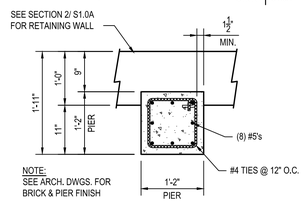
SHEET NO.
S1.0



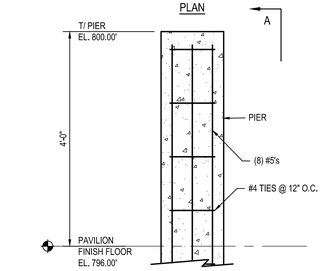
SITE RETAINING WALL SCHEDULE								
"H" ft.-in.	"w" in	"a" ft.-in.	"b" ft.-in.	"c" ft.-in.	"d" ft.-in.	"t" in	V-BARS	X-BARS
2'-0"	12"	1'-0"	----	----	3'-0"	12"	#5 @ 12"o.c.	#5 @ 12"o.c.
4'-0"	12"	1'-0"	----	----	3'-0"	12"	#5 @ 12"o.c.	#5 @ 12"o.c.
6'-0"	12"	1'-0"	----	----	5'-6"	12"	#5 @ 12"o.c.	#5 @ 12"o.c.
8'-0"	12"	1'-0"	----	----	7'-6"	16"	#5 @ 12"o.c.	#6 @ 12"o.c.
10'-0"	12"	1'-6"	----	----	9'-0"	16"	#6 @ 12"o.c.	#6 @ 12"o.c.

NOTE:
1) PROVIDE EXPANSION JOINTS IN WALL AT 24'-0" ON CENTER.
2) MINIMUM BAR LAP IS 40 DIAMETERS

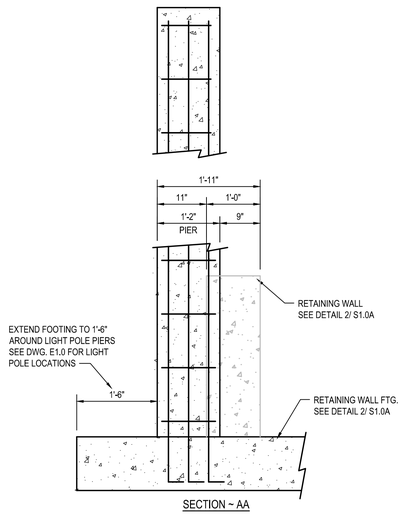
SECTION 2 / S1.0A
NTS



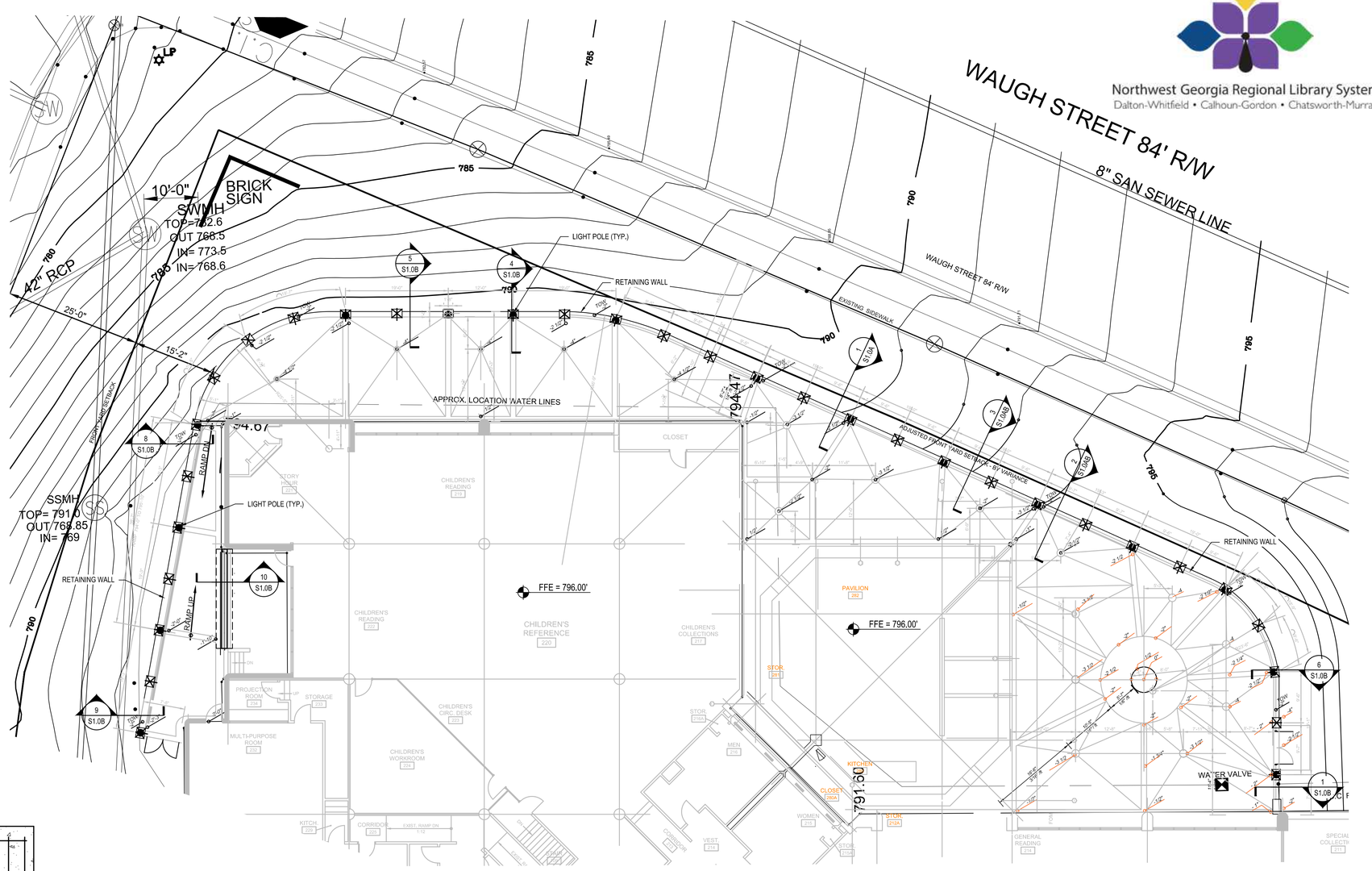
PLAN



3 / S1.0A RETAINING WALL PIER
SCALE: 3/4"=1'-0"



SECTION ~ AA



NORTH

1 / S1.0A RETAINING WALL PLAN
SCALE: 3/32"=1'-0"

BUILDING FINISH FLOOR ELEVATION = 796.00
TOW = TOP OF PRECAST COPING AT -2", EL. 795.83
TOP OF RETAINING WALL AT -1'-0", EL. 795.00
TOP OF PIERS AT +4'-0", EL. 800.00

NOTE:
REFER TO ARCHITECTURAL DRAWINGS FOR LIGHT POLE LOCATIONS

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RETAINING WALL PLAN

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S1.0A

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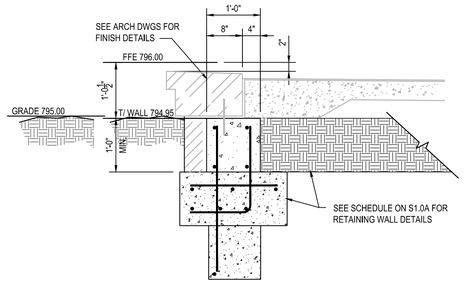

1 S1.0B RETAINING WALL SECTION
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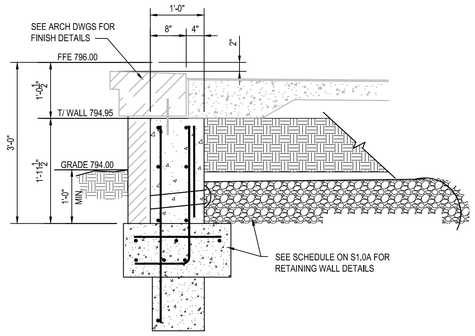

2 S1.0B RETAINING WALL SECTION
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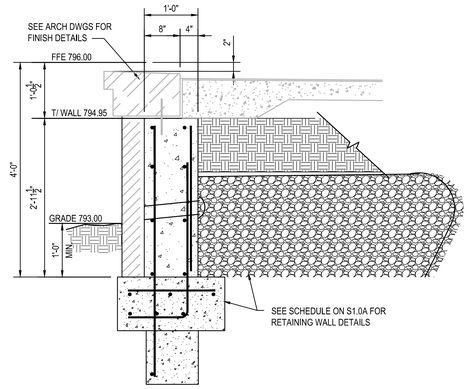

3 S1.0B RETAINING WALL SECTION
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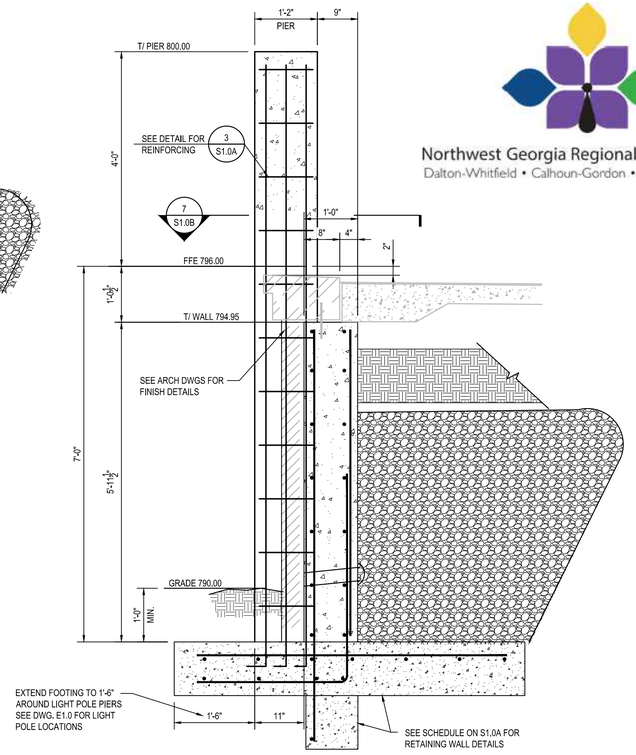
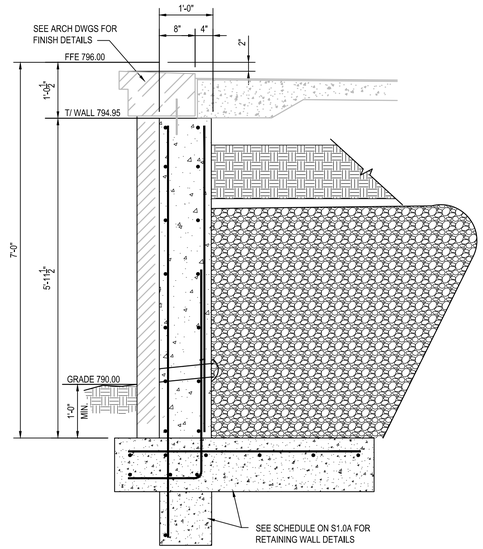
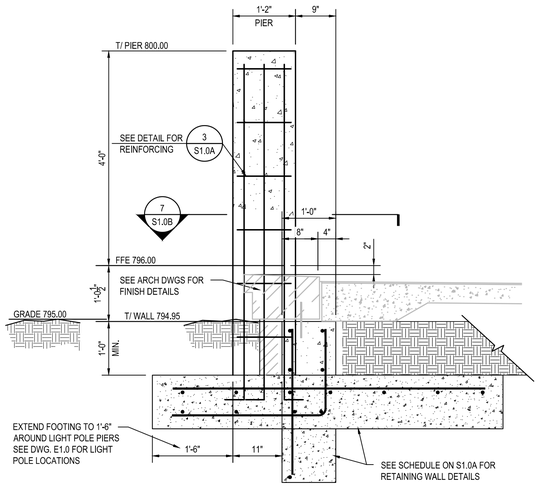


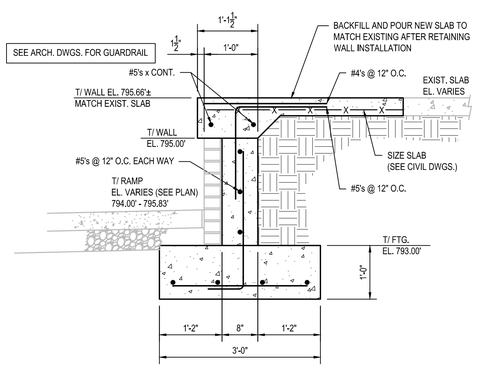
4 S1.0B RETAINING WALL PIER SECTION
SCALE: 3/4"=1'-0"



5 S1.0B RETAINING WALL SECTION
SCALE: 3/4"=1'-0"



6 S1.0B RETAINING WALL PIER SECTION
SCALE: 3/4"=1'-0"



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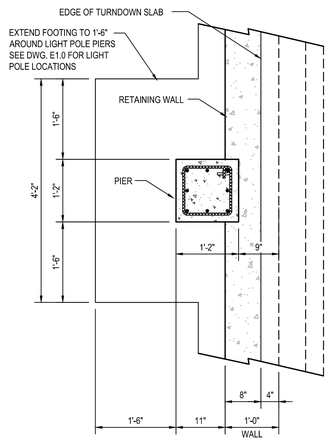
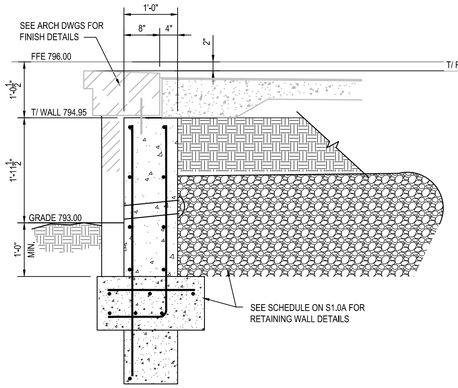
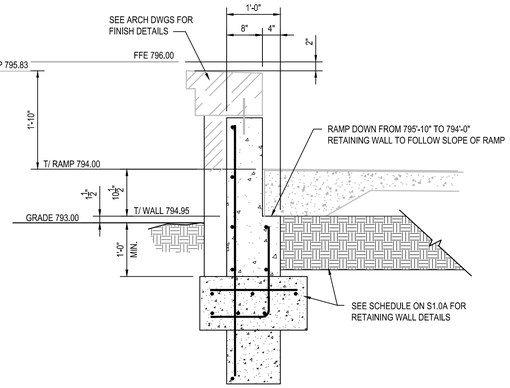
SCALE: 3/4"=1'-0"



7 S1.0B PIER PLAN
SCALE: 3/4"=1'-0"



8 S1.0B RETAINING WALL SECTION @ TOP OF RAMP
SCALE: 3/4"=1'-0"



9 S1.0B RETAINING WALL SECTION @ BOTTOM OF RAMP
SCALE: 3/4"=1'-0"

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RETAINING WALL SECTIONS

SHEET NO.
S1.0B

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Chattanooga, Tennessee 37404
PH: (423)698-6675
MAA JN: 24133

STRUCTURAL LEGEND:

(?) COLUMN/FOOTING DESIGNATION - SEE COLUMN/FOOTING SCHEDULE ON S0.1.

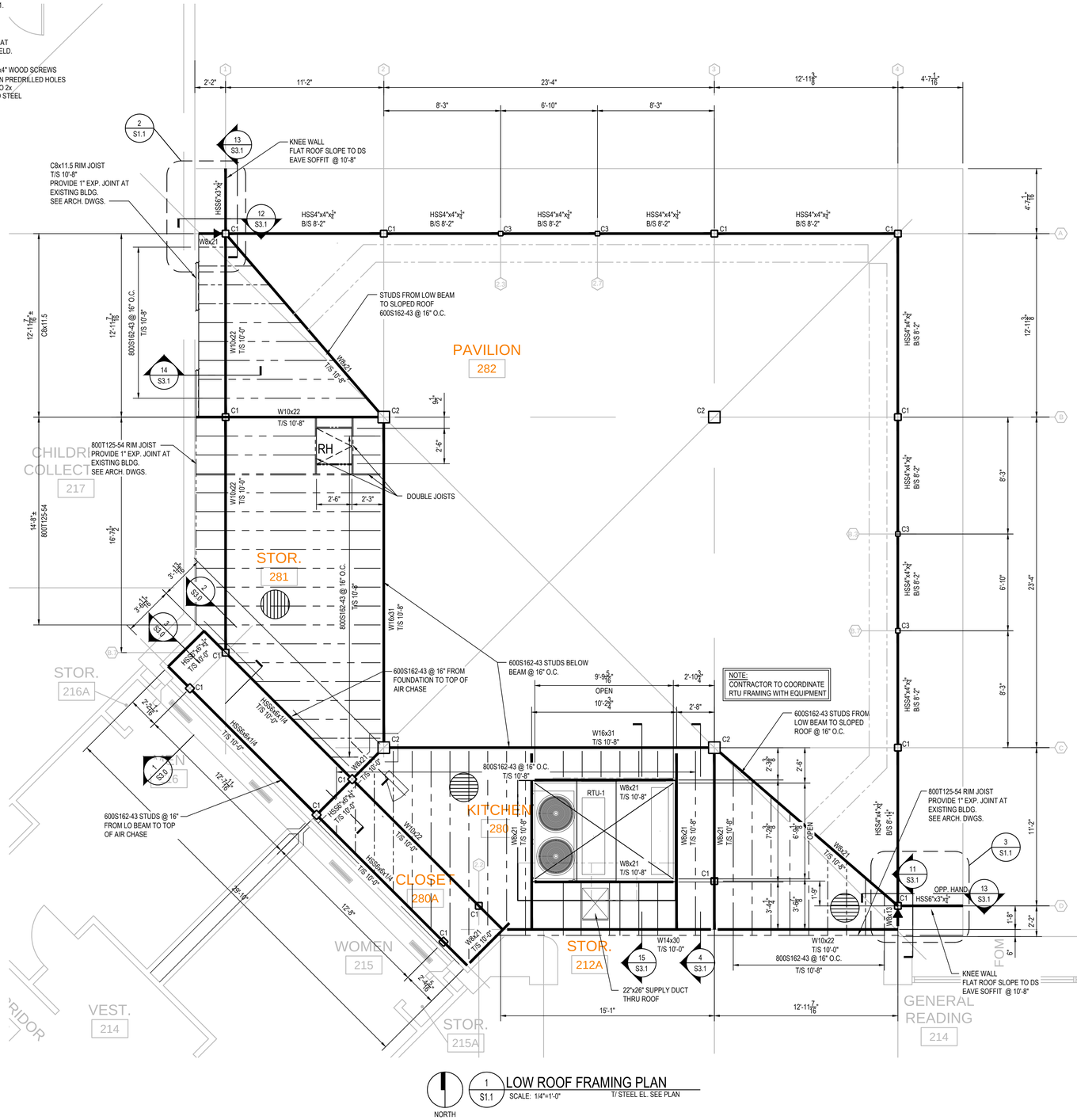
T/J INDICATES TOP OF JOIST ELEVATION ABOVE FINISHED FLOOR.

15/32" RATED SHEATHING w/ 8d NAILS AT 6" o.c. AT SHEET PERIMTER, 12" o.c. FIELD.
3x6 #2 SYP T&G ATTACH TO FRAMING
40d TOENAIL AT SUPPORTS w. (2) #14x4" WOOD SCREWS
8" SPIKES AT 30" o.c. HORIZONTALLY IN PREDRILLED HOLES
(2) #8x2.5" WOOD SCREWS FROM 2x TO 2x
(2) SIMPSON PDPAWL 250 FROM 2x TO STEEL

SW7 INDICATES SHEAR WALL SEE SCHEDULE ON S0.1

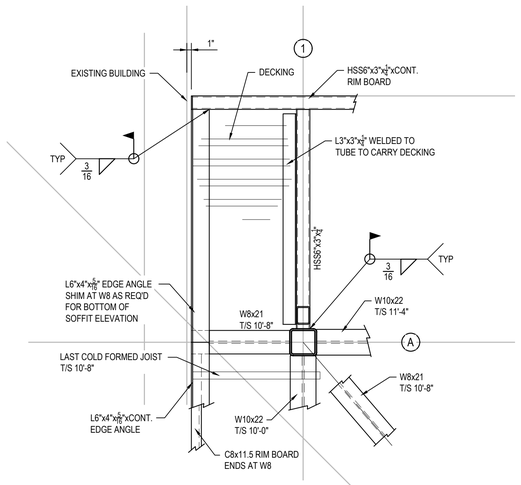
INDICATES FULL-PEN MOMENT CONNECTION

15/32" RATED SHEATHING WITH #8 SCREWS AT 6" O.C. AT SUPPORTS

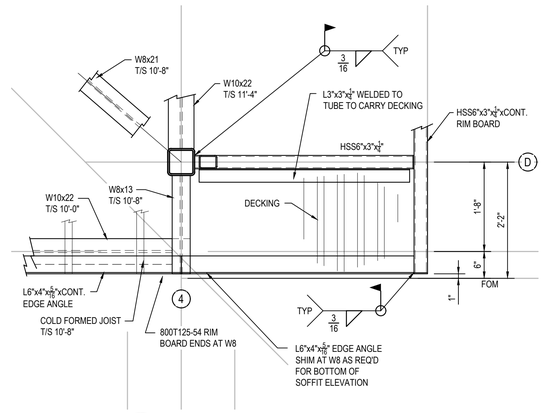


1 S1.1 LOW ROOF FRAMING PLAN
SCALE: 1/4"=1'-0" T/ STEEL EL. SEE PLAN

NORTH



2 S1.1 ROOF OVERFLOW FRAMING PLAN
SCALE: 3/4"=1'-0" NW CORNER



3 S1.1 ROOF OVERFLOW FRAMING PLAN
SCALE: 3/4"=1'-0" SE CORNER

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LOW ROOF FRAMING PLAN

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S1.1

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STRUCTURAL LEGEND:

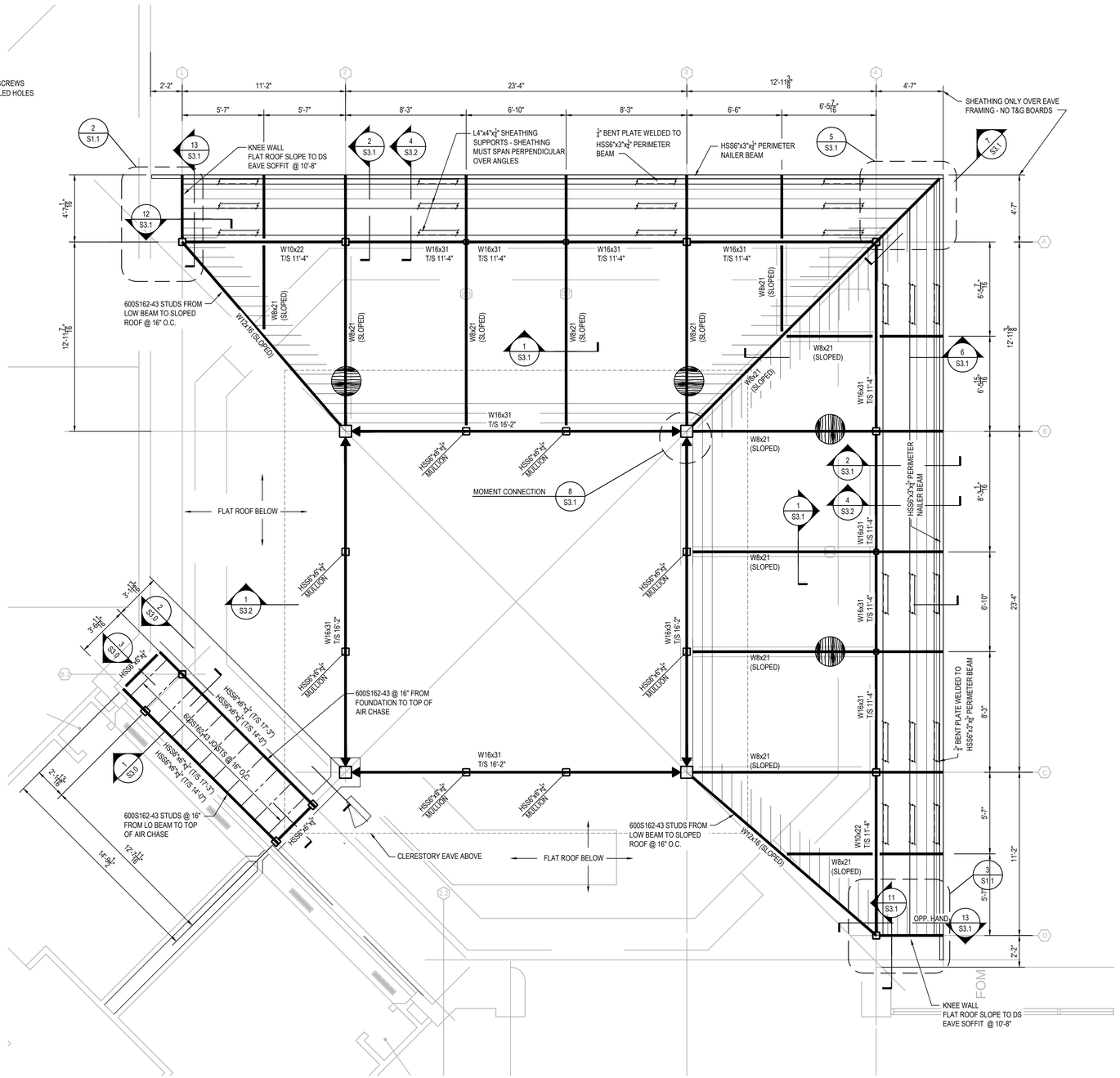
(?) COLUMN/FOOTING DESIGNATION - SEE COLUMN/FOOTING SCHEDULE ON S0.1.

T/J INDICATES TOP OF JOIST ELEVATION ABOVE FINISHED FLOOR.

15/32" RATED SHEATHING w/ 8d NAILS AT 6" o.c. AT SHEET PERIMTER, 12" o.c. FIELD.
3x6 #2 SYP T&G ATTACH TO FRAMING
40# TOENAIL AT SUPPORTS w. (2) #14x4" WOOD SCREWS
8" SPIKES AT 30" o.c. HORIZONTALLY IN PREDRILLED HOLES
(2) #8x2.5" WOOD SCREWS FROM 2x TO 2x
(2) SIMPSON PDPAWL 250 FROM 2x TO STEEL

SW? INDICATES SHEAR WALL SEE SCHEDULE ON S0.1

INDICATES FULL-PEN MOMENT CONNECTION



1 S1.2 HIGH ROOF FRAMING PLAN
SCALE: 1/4"=1'-0" T/ STEEL EL. (SEE PLAN)

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HIGH ROOF FRAMING PLAN

SHEET NO.
S1.2

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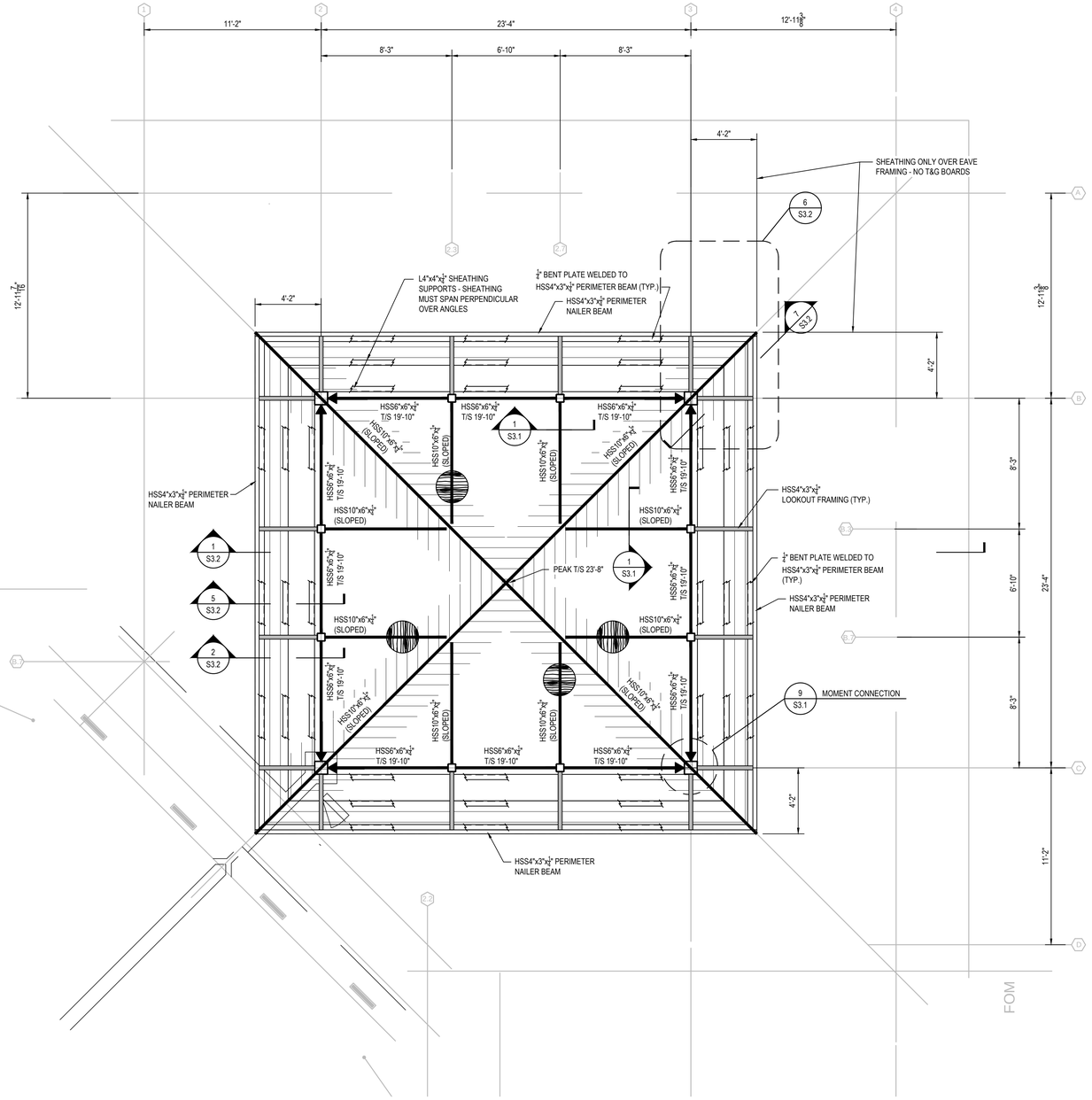

1 S1.3 CLERESTORY FRAMING PLAN
SCALE: 1/4"=1'-0"
T/ STEEL EL. 19'-10" U.N.O.

NORTH

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STRUCTURAL LEGEND:

(7) COLUMN/FOOTING DESIGNATION - SEE COLUMN/FOOTING SCHEDULE ON S0.1.

T/J INDICATES TOP OF JOIST ELEVATION ABOVE FINISHED FLOOR.

15/32" RATED SHEATHING w/ 8d NAILS AT 6" o.c. AT SHEET PERIMTER, 12" o.c. FIELD.
3x6 #2 SYP T&G ATTACH TO FRAMING
40d TOENAIL AT SUPPORTS w. (2) #14x4" WOOD SCREWS
8" SPIKES AT 30" o.c. HORIZONTALLY IN PREDRILLED HOLES
(2) #8x2.5" WOOD SCREWS FROM 2x TO 2x
(2) SIMPSON PDPAWL 250 FROM 2x TO STEEL

SW7 INDICATES SHEAR WALL SEE SCHEDULE ON S0.1

INDICATES FULL-PEN MOMENT CONNECTION

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CLERESTORY FRAMING PLAN

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S1.3

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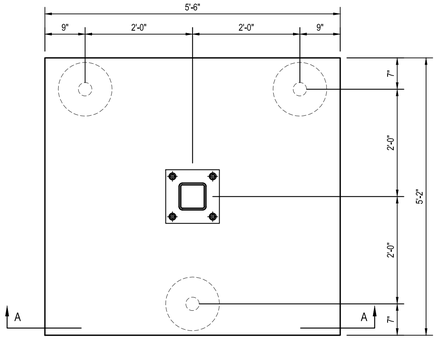
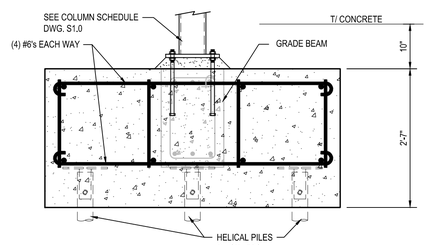
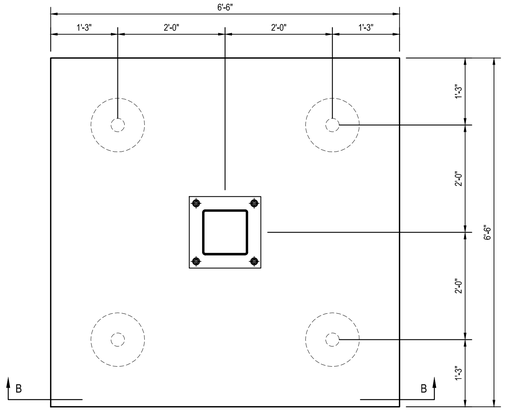


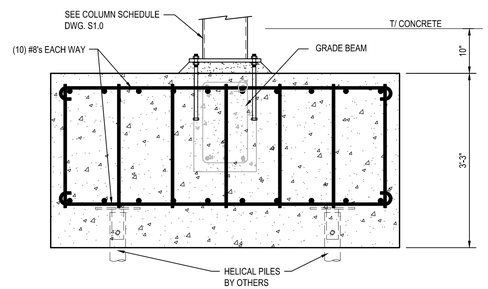
1 S2.0 PILE CAP PLAN A
SCALE: 3/4"=1'-0"
NORTH



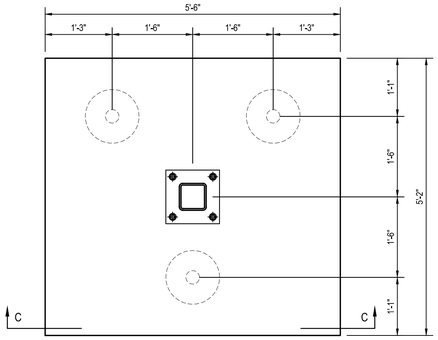
SECTION ~ AA



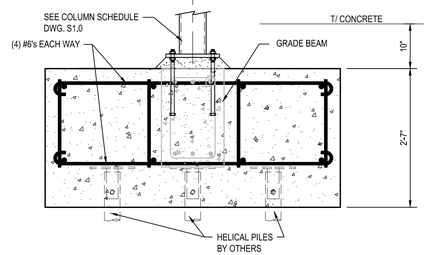
2 S2.0 PILE CAP PLAN B
SCALE: 3/4"=1'-0"
NORTH



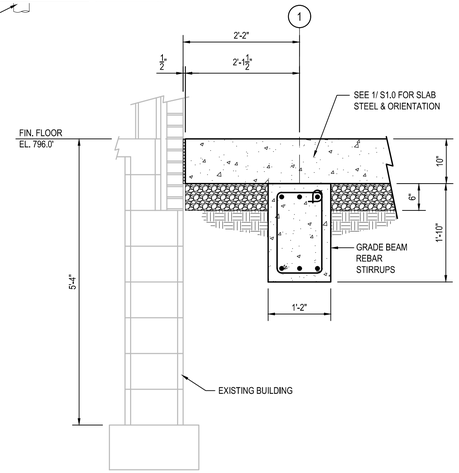
SECTION ~ BB



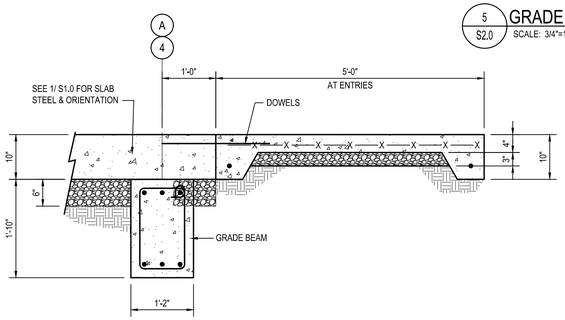
3 S2.0 PILE CAP PLAN C
SCALE: 3/4"=1'-0"
NORTH



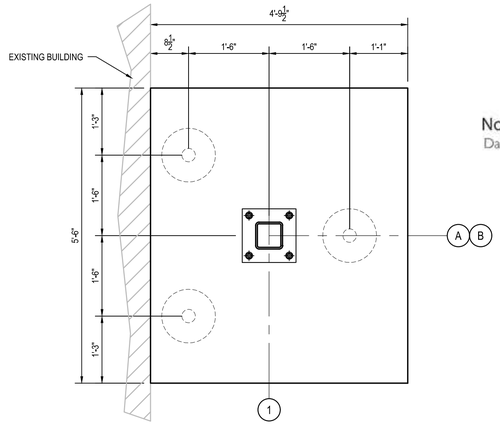
SECTION ~ CC



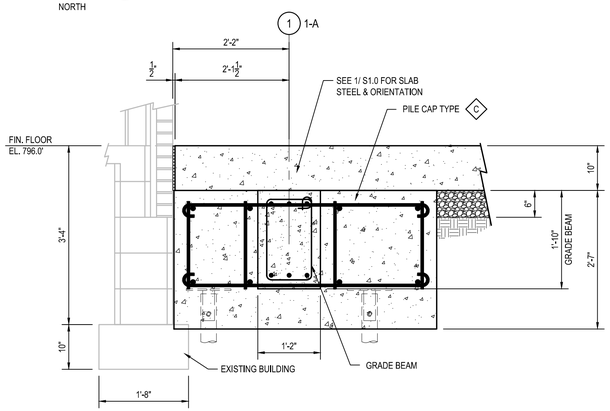
5 S2.0 GRADE BEAM SECTION
SCALE: 3/4"=1'-0"



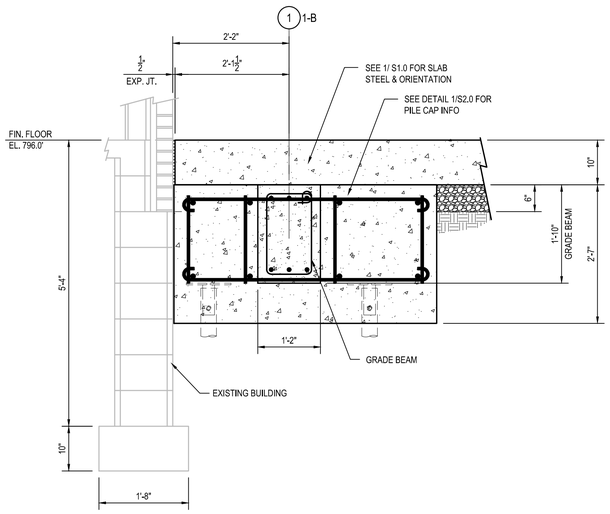
6 S2.0 GRADE BEAM SECTION
SCALE: 3/4"=1'-0"



4 S2.0 PILE CAP PLAN C
SCALE: 3/4"=1'-0"
NORTH



4A S2.0 PILE CAP SECTION
SCALE: 3/4"=1'-0"



4B S2.0 PILE CAP SECTION
SCALE: 3/4"=1'-0"

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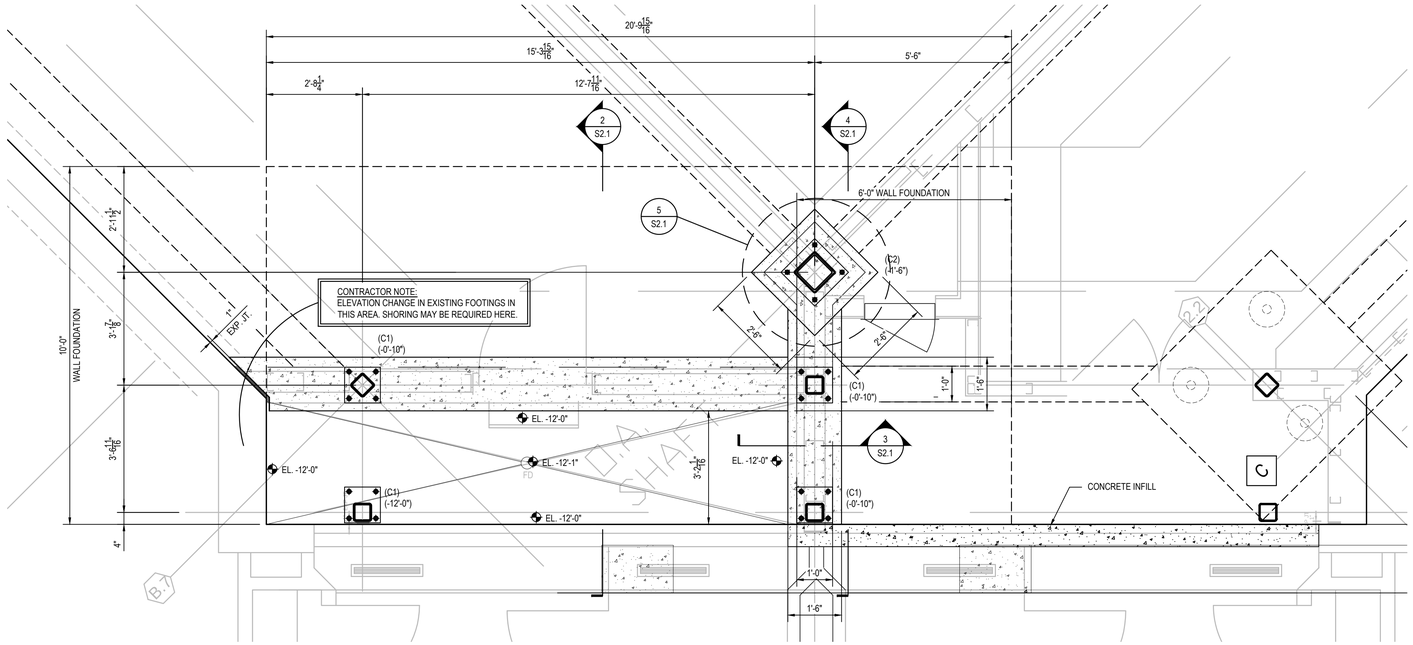
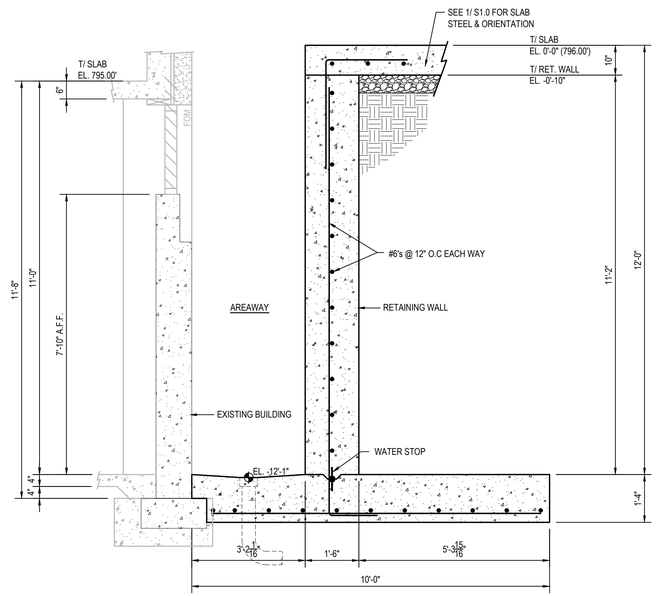


NORTH

1 / S2.1 AREAWAY FOUNDATION PLAN

SCALE: 1/2"=1'-0"

NOTE:
SEE DWG. S1.0 FOR COLUMN SCHEDULE



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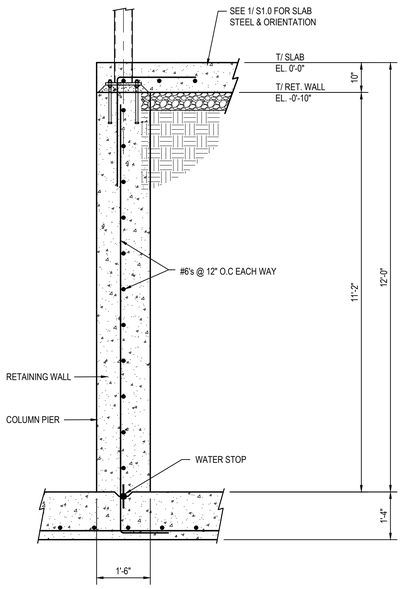

3 / S2.1 AREAWAY FOUNDATION SECTION

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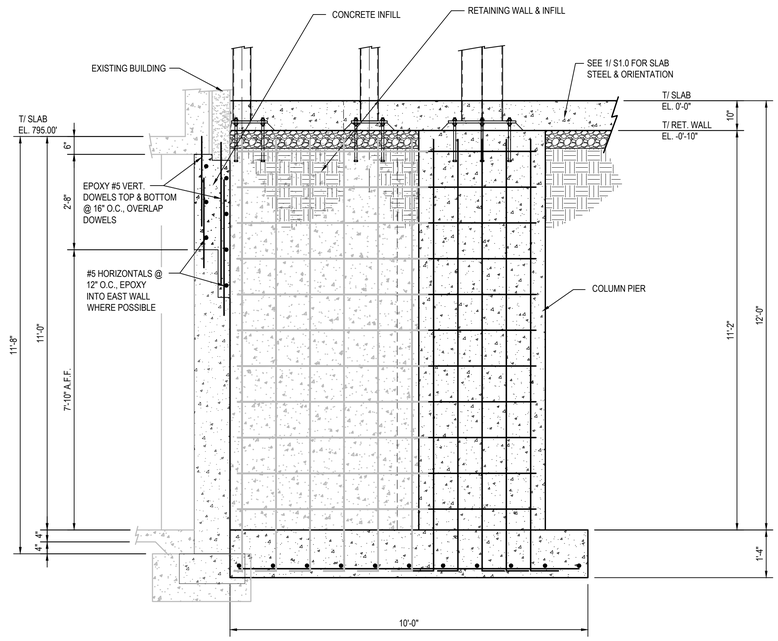

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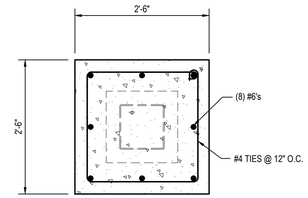

5 / S2.1 COLUMN PIER

SCALE: 3/4"=1'-0"

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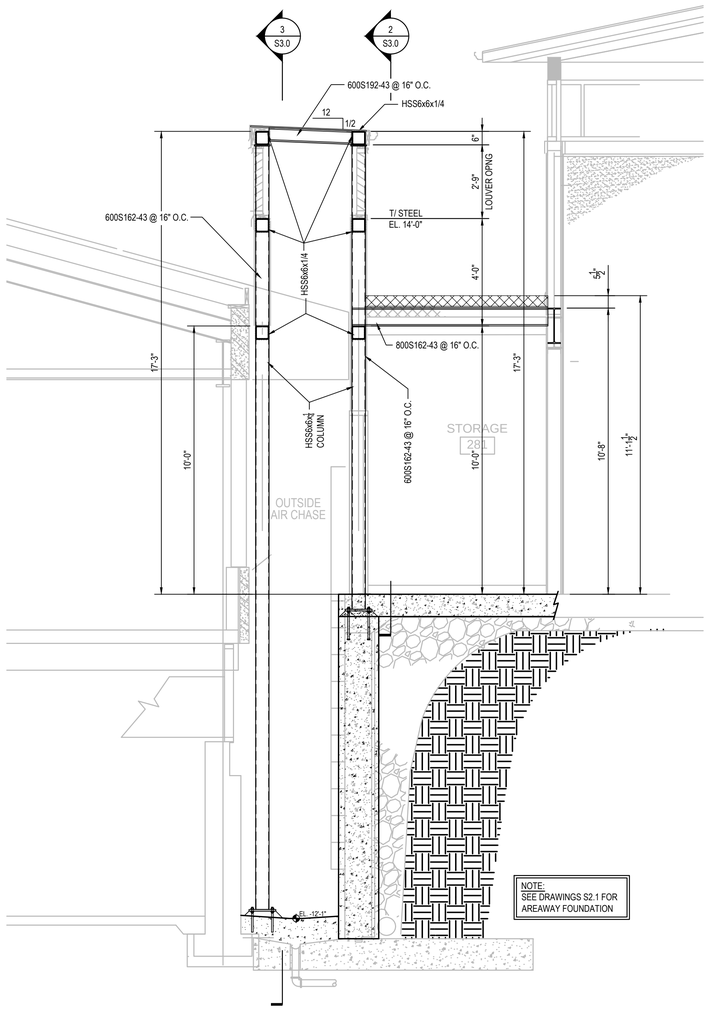
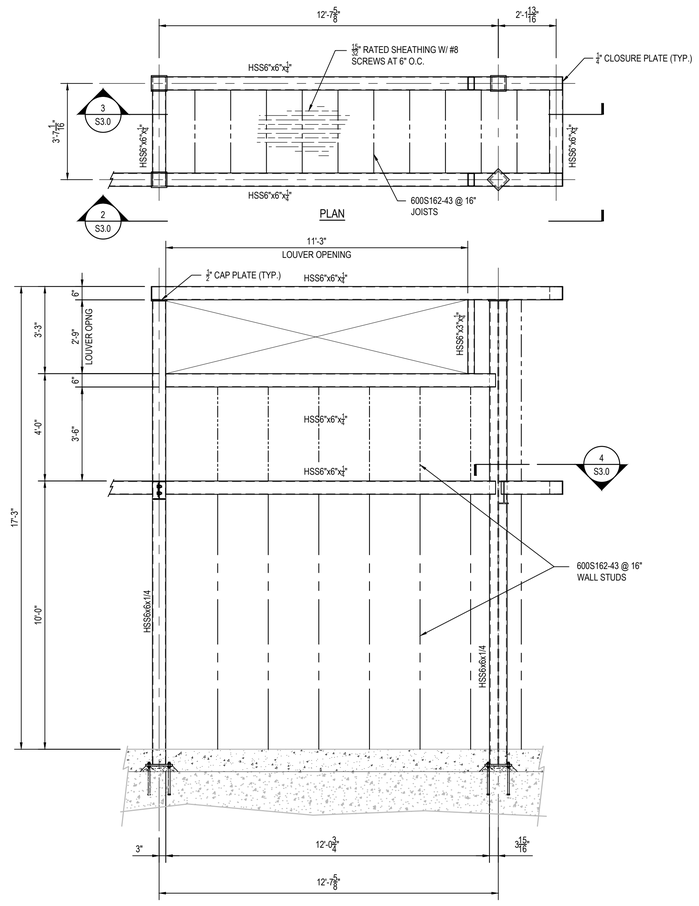
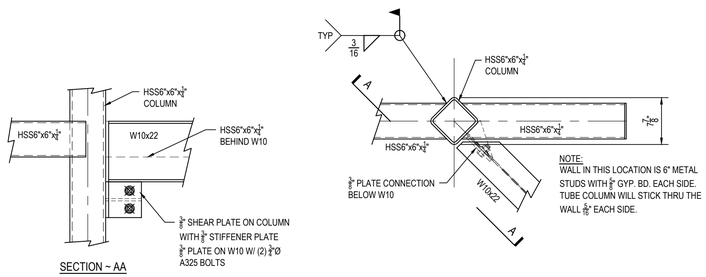


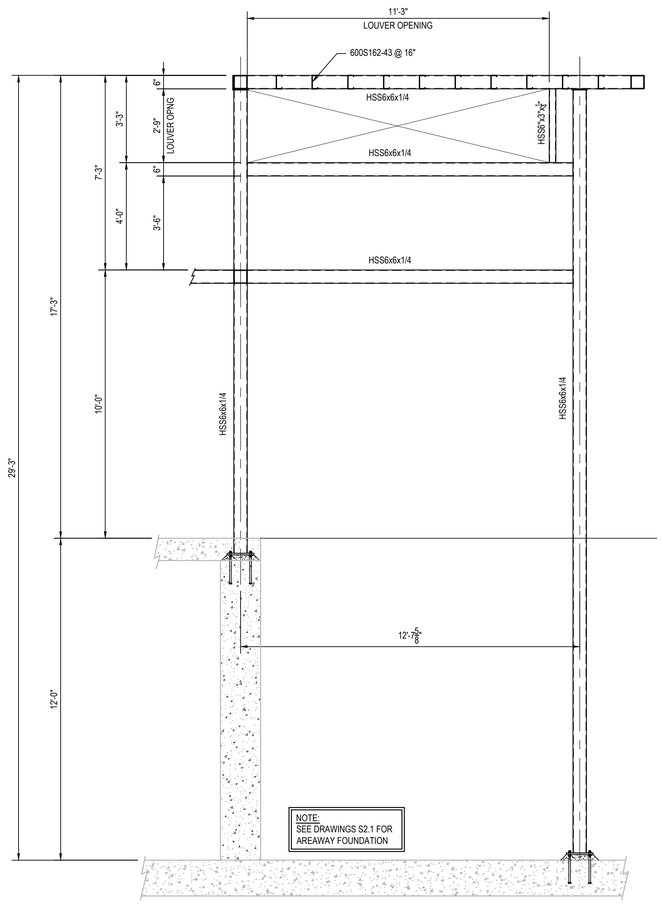
1 S3.0 AREAWAY FRAMING SECTION
SCALE: 3/8"=1'-0"



2 S3.0 AREAWAY FRAMING SECTION
SCALE: 3/8"=1'-0"



4 S3.0 CONNECTION DETAIL
SCALE: 1"=1'-0"



3 S3.0 AREAWAY FRAMING SECTION
SCALE: 3/8"=1'-0"

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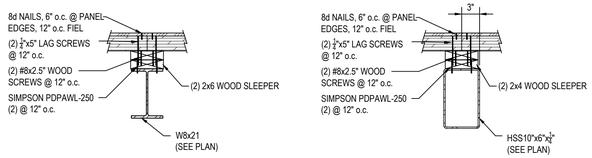
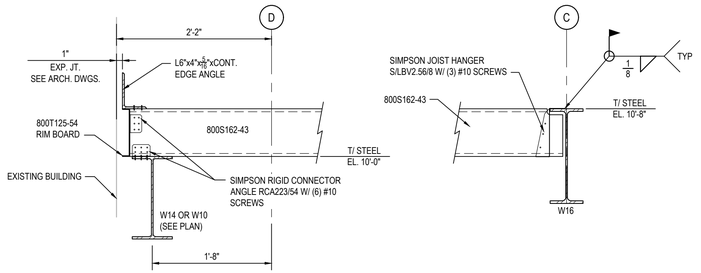
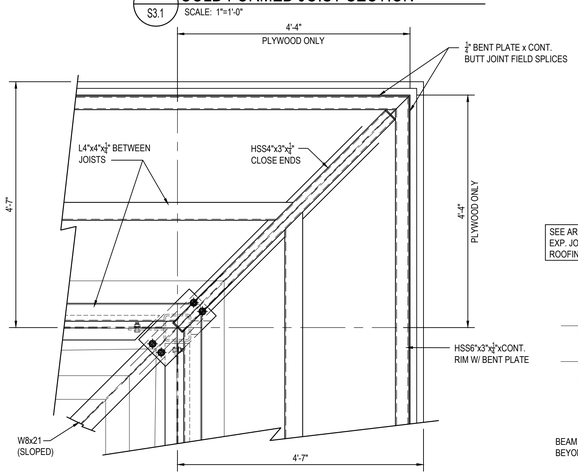


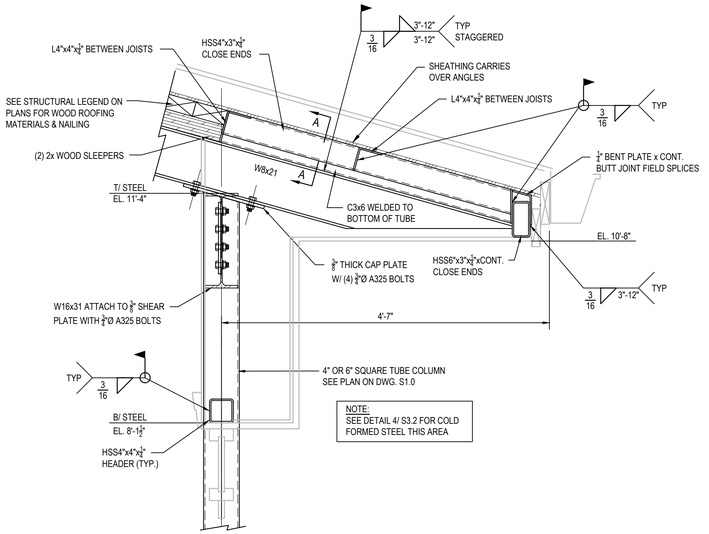
1 S3.1 **WOOD ROOFING DETAIL**
SCALE: 1"=1'-0"



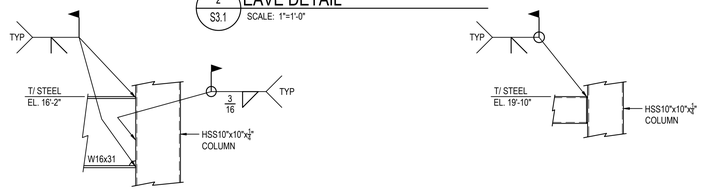
4 S3.1 **COLD FORMED JOIST SECTION**
SCALE: 1"=1'-0"



5 S3.1 **HIGH ROOF CORNER PLAN**
SCALE: 3/4"=1'-0"

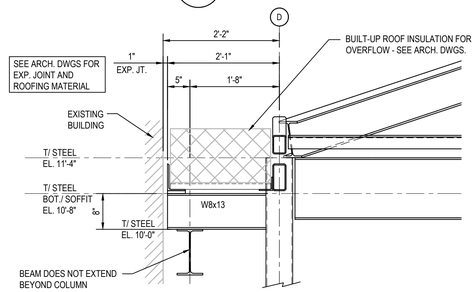


2 S3.1 **EAVE DETAIL**
SCALE: 1"=1'-0"

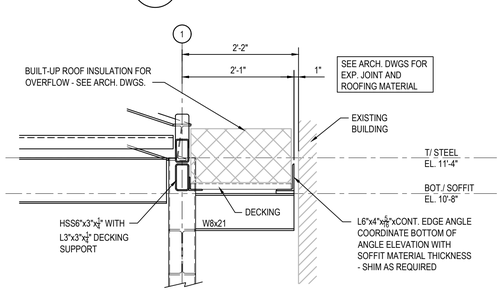


8 S3.1 **MOMENT CONNECTION - WIDE FLANGE**
SCALE: 3/4"=1'-0"

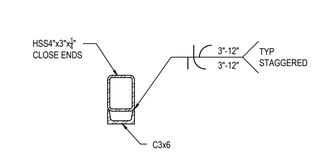
9 S3.1 **MOMENT CONNECTION - TUBE STEEL**
SCALE: 3/4"=1'-0"



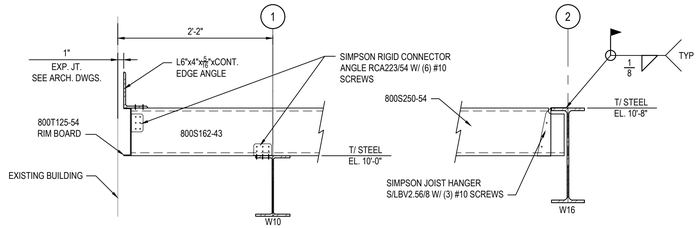
11 S3.1 **EAST ROOF OVERFLOW**
SCALE: 3/4"=1'-0" (LOOKING WEST)



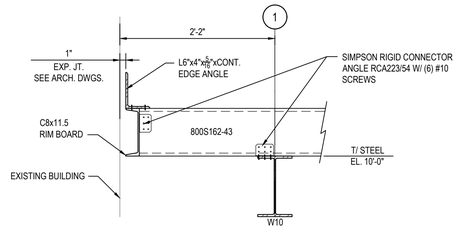
12 S3.1 **NORTH ROOF OVERFLOW**
SCALE: 3/4"=1'-0" (LOOKING SOUTH)



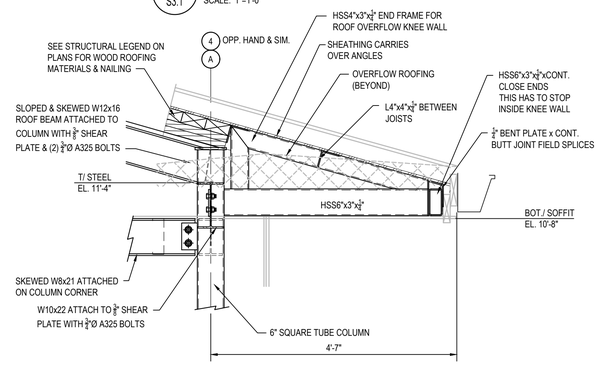
SECTION ~ AA



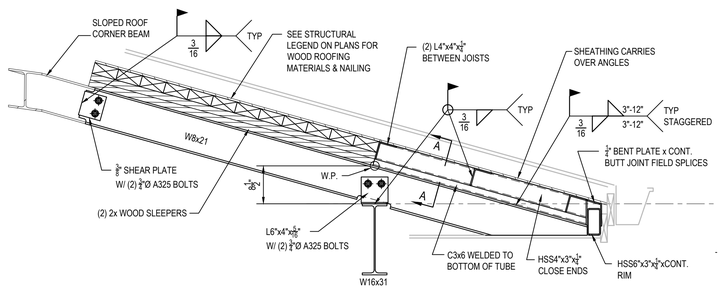
3 S3.1 **COLD FORMED JOIST SECTION**
SCALE: 1"=1'-0"



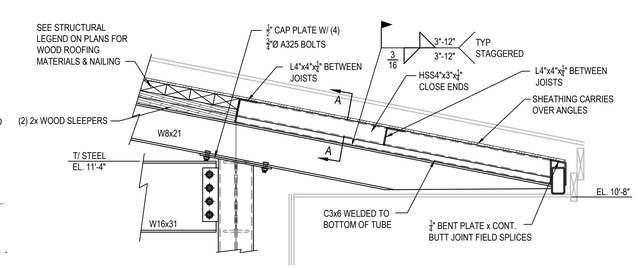
14 S3.1 **COLD FORMED JOIST SECTION**
SCALE: 1"=1'-0"



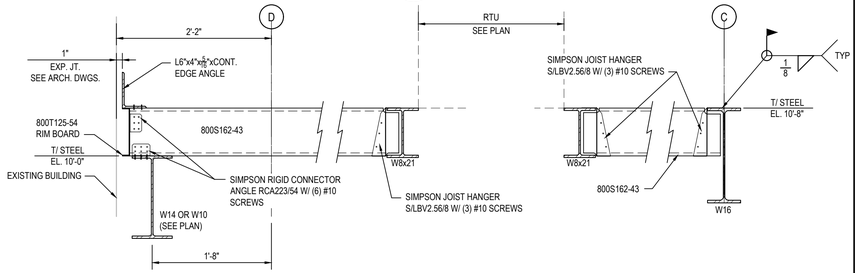
13 S3.1 **ROOF OVERFLOW SECTION**
SCALE: 3/4"=1'-0"



6 S3.1 **JOIST OVER ROOF BEAM DETAIL**
SCALE: 3/4"=1'-0"



7 S3.1 **HIGH ROOF CORNER SECTION**
SCALE: 3/4"=1'-0" DEVELOPED VIEW



15 S3.1 **COLD FORMED JOIST SECTION @ RTU**
SCALE: 1"=1'-0"

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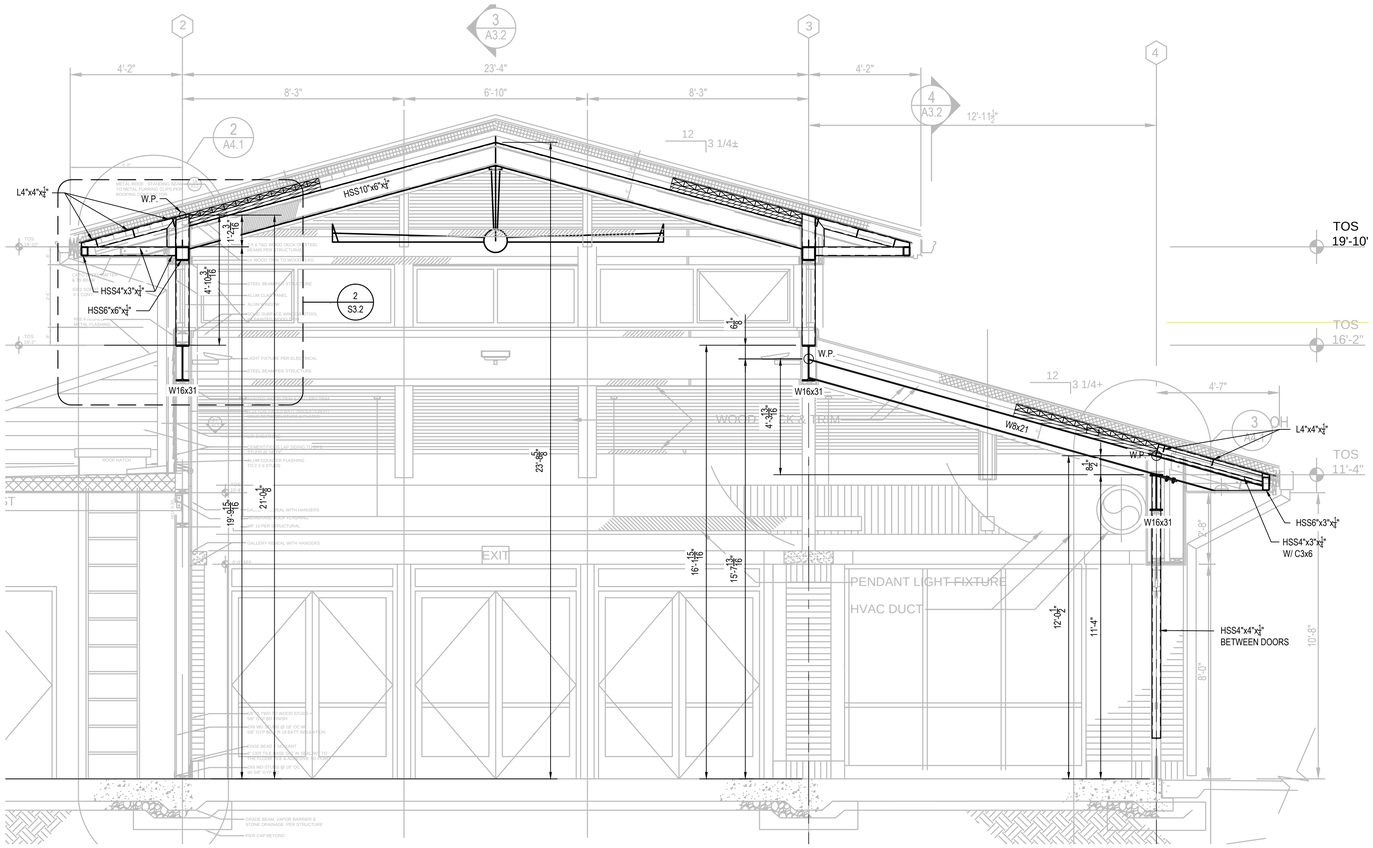
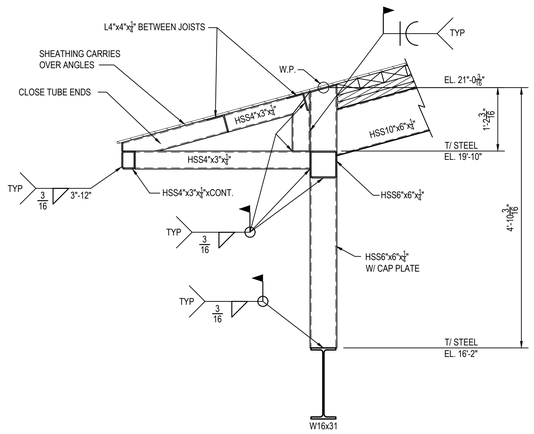
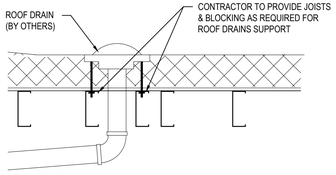


1 S3.2 BUILDING SECTION - JOIST WORKPOINTS
SCALE: 3/8"=1'-0"



2 S3.2 CLERESTORY EAVE DETAIL
SCALE: 3/4"=1'-0"



3 S3.2 ROOF DRAIN SUPPORT
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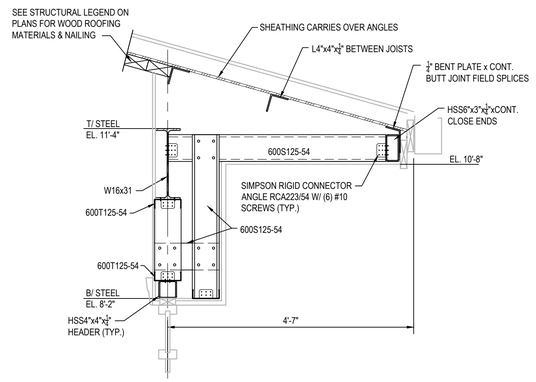
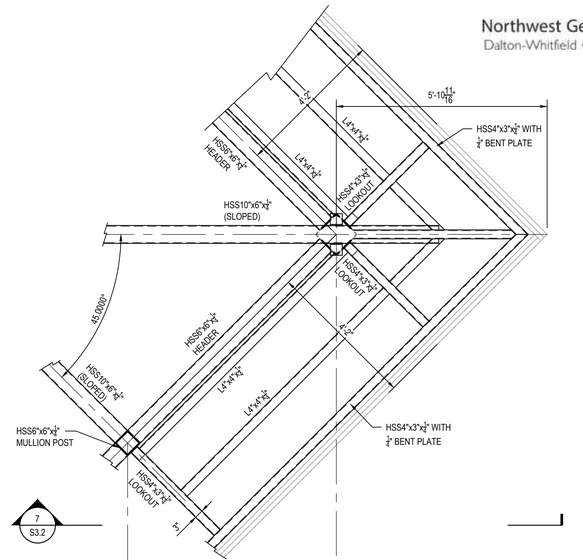
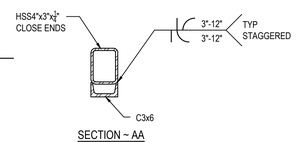


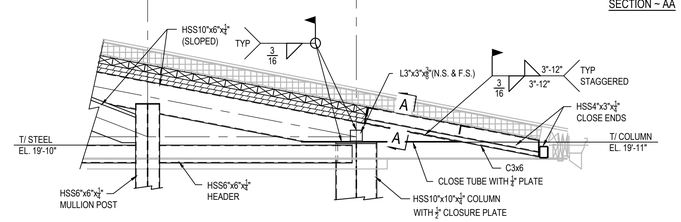
4 S3.2 SOFFIT / STOREFRONT DETAIL
SCALE: 3/4"=1'-0" ALSO SEE 2/ S3.1



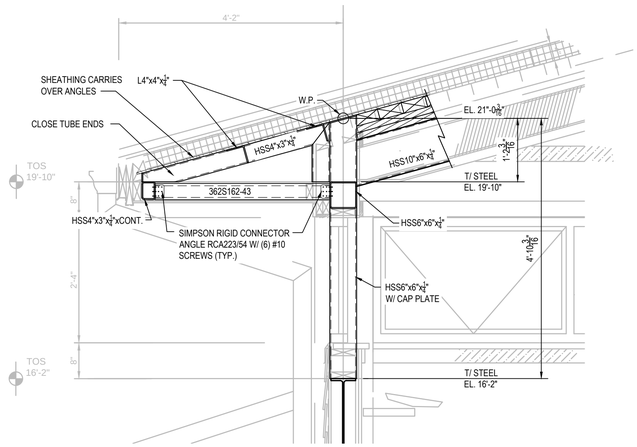
6 S3.2 CLERESTORY ROOF CORNER PLAN
SCALE: 1/2"=1'-0"



SECTION ~ AA



7 S3.2 CLERESTORY ROOF CORNER DEVELOPED SECTION
SCALE: 1/2"=1'-0"



5 S3.2 CLERESTORY SOFFIT DETAIL
SCALE: 3/4"=1'-0"

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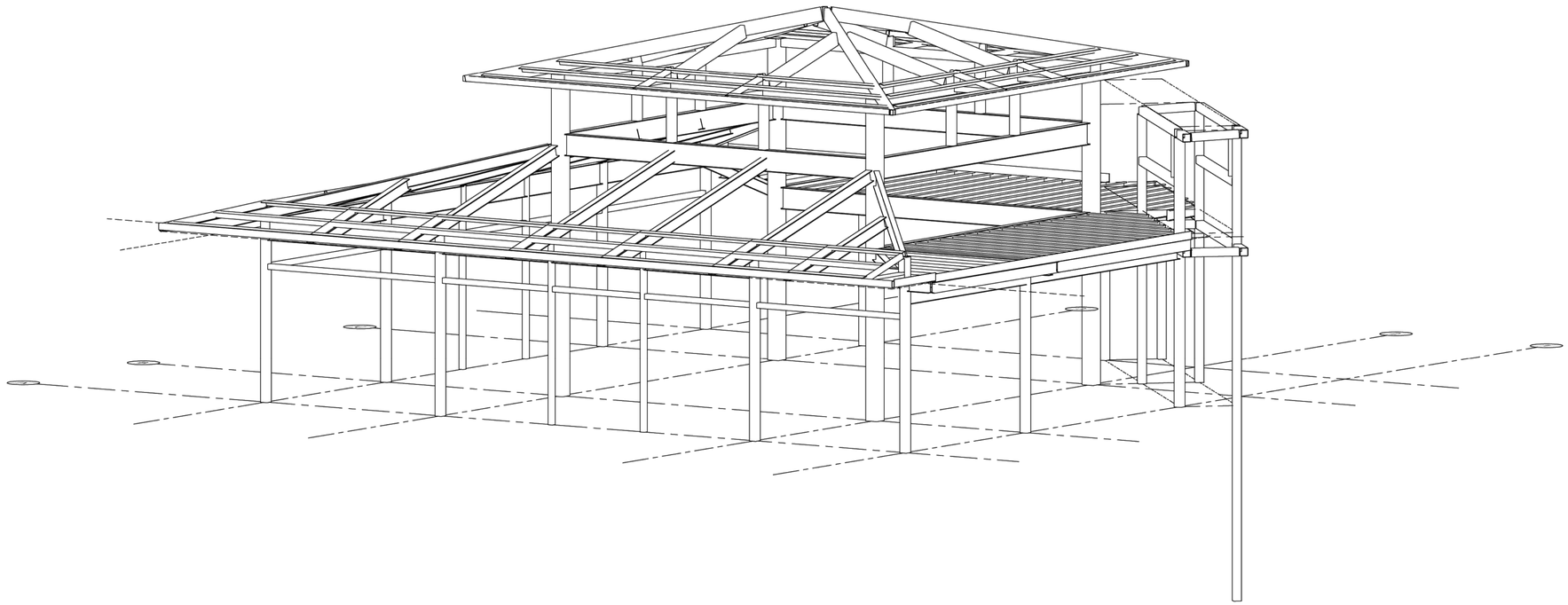

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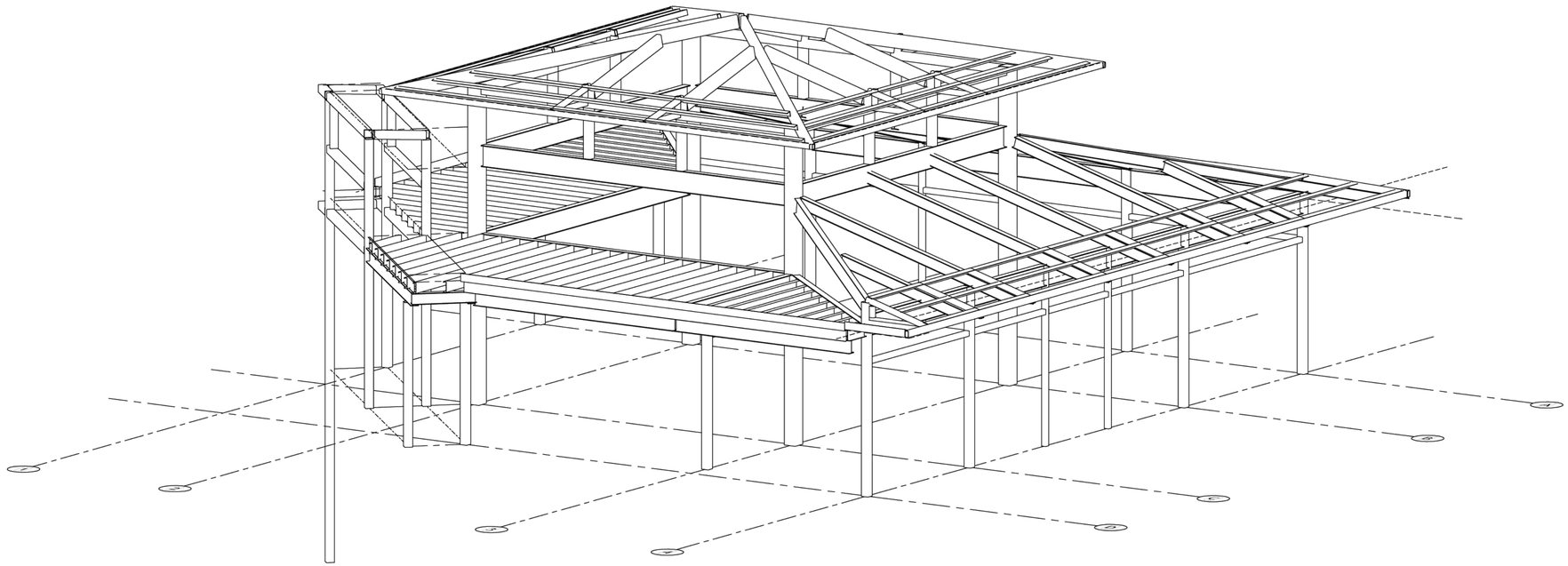

1 / S4.1 SOUTHEAST ISOMETRIC
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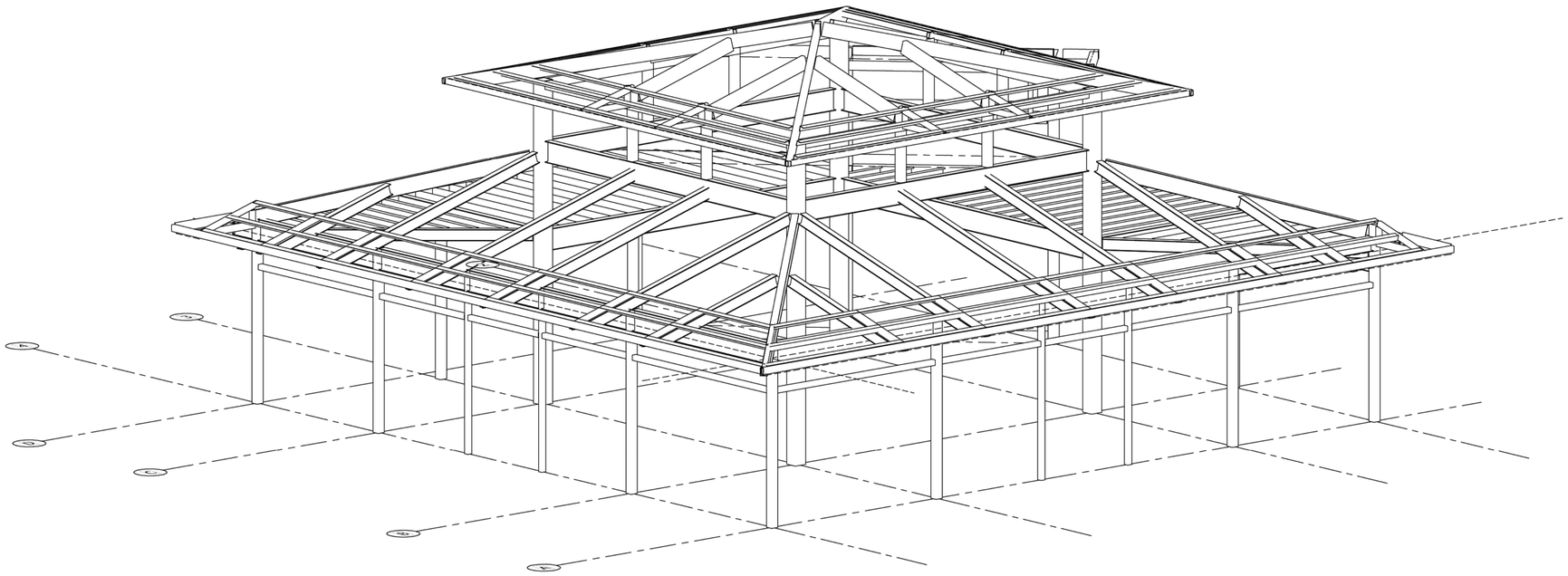

1 S4.2 NORTHEAST ISOMETRIC
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